TOWN OF GOLDEN BEACH, FLORIDA

RESOLUTION NO. 2639.19

A RESOLUTION OF THE TOWN OF GOLDEN BEACH, FLORIDA, AUTHORIZING AND APPROVING A VARIANCE REQUEST FOR THE PROPERTY LOCATED AT 399 GOLDEN BEACH DR., GOLDEN BEACH, FLORIDA 33160 TO PERMIT AN ENTRANCE CANOPY TO ENCROACH AT VARYING FRONT SETBACKS, FROM 26.05' AT THE CURVE OF THE NORTH FRONT PROPERTY LINE TO 33.27' FROM THE SOUTH FRONT PROPRETY LINE, INSTEAD OF THE 35 FOOT FRONT SETBACK OUTLINED IN THE CODE.

WHEREAS, the applicants, Galsky Construction Enterprises 2 LLC, ("the applicant"), filed a Petition for Variances/exceptions, from Town Code Section 66.69-2 Zone Two, (f) front yard setback: (2) Lots fronting east side of Golden Beach Drive. Lots with Frontage on the east side of Golden Beach Drive shall provide front yard Setbacks as follows: a) For Full Size Lots, no building or part thereof, including garages, shall be erected nearer than 35 feet from the west lot line thereof.

WHEREAS, the applicant's request is to allow the front entrance Canopy/Trellis structure to encroach at varying front setbacks; from a front setback of 26.05' at the curve of the north front property line to front setback of 33.27' from the south front property line. Instead of the 35 foot setback outlined in the code.

WHEREAS, these variances and exceptions are for the property at 399 Golden Beach Dr., Golden Beach, FL. 33160 (Golden Beach Section "E", N 19.50' of lot 39 and all of Lot 40, Block 4, as recorded in PB 8-122, of the Public Records of Miami-Dade County, (Folio No. 19-1235-005-0330 (the "Property") and;

WHEREAS, the Town's Building Regulation Advisory Board held an advertised public hearing on the Petition for Variance/Exception and recommended approval of the request for the entrance canopy, for approval by the Town Council; and,

WHEREAS, a public hearing of the Town Council was advertised and held, as required by law, and all interested parties were given an opportunity to be heard; and

WHEREAS, the Town Council having considered the evidence presented, finds that the Petition of Variance meets the criteria of the applicable codes and ordinances to the extent the application is granted herein.

NOW, THEREFORE, BE IT RESOLVED BY THE TOWN COUNCIL OF THE TOWN OF GOLDEN BEACH, FLORIDA, AS FOLLOWS:

Section 1. Recitals Adopted. Each of the above stated recitals are hereby adopted and confirmed.

Section 2. Approval. The Petition for Variance to permit each of the requested variances/exception is hereby granted.

Section 3. Conditions. The Petition for Exception/Variance as granted is subject to the following conditions:

(1) Applicant shall record a certified copy of this Resolution in the public records of Miami-Dade County; and the construction shall be completed substantially in accordance with those certain plan pages SD-0.0 through SD 2.2, dated May 14, 2019, Gonzalo Paz, PE, #60734, Eastern Engineering Group, and the Sketch of Boundary Survey, prepared by John Ibarra & Associates, Inc., Professional Land Surveyors, dated 7/17/2019, for the property located at 399 Golden Beach Dr., Golden Beach, FL. 33160

Section 4. Implementation. That the Building and Zoning Director is hereby directed to make the necessary notations upon the maps and records of the Town of Golden Beach Building and Zoning Department and to issue all permits in accordance with the terms and conditions of this Resolution. A copy of this Resolution shall be attached to the building permit application documents.

Section 5. Effective Date. This Resolution shall be effective immediately upon adoption.

Sponsored by Administration.

The Motion to adopt the foregoing Resolution was offered by Councilmember Lusskin, seconded by Councilmember Bernstein, and on roll call the following vote ensued:

Mayor Glenn Singer	Aye
Vice Mayor Kenneth Bernstein	Aye
Councilmember Judy Lusskin	Aye
Councilmember Jaime Mendal	Aye
Councilmember Bernard Einstein	Aye

PASSED AND ADOPTED by the Town Council of the Town of Golden Beach, Florida, this 17th day September, 2019

MAYOR GLENN SINGER

ATTEST:

LISSETTE PEREZ
TOWN CLERK

APPROVED AS TO FORM
AND LEGAL SUFFICIENCY:

STEPHEN J. HELFMAN
TOWN ATTORNEY

TOWN OF GOLDEN BEACH

One Golden Beach Drive
Golden Beach, FL 33160

M E M O R A N D U M

Date: September 17, 2019

Item Number: 6

To: Honorable Mayor Glenn Singer &
Town Council Members

From: Alexander Diaz,
Town Manager

Subject: **Resolution No. 2639.19 – Variance Request for 399 Golden Beach Drive, Golden Beach, FL 33160 (Entrance Canopy)**

Recommendation:

It is recommended that the Town Council allow the applicant the opportunity to seek approval of the variance request presented in Resolution No. 2639.19.

Background and History:

Town Code Section 66.69-2 Zone Two, (f) front yard setback:
(2) Lots fronting east side of Golden Beach Drive. Lots with Frontage on the east side of Golden Beach Drive shall provide front yard Setbacks as follows:
a) For Full Size Lots, no building or part thereof, including garages, shall be erected nearer than 35 feet from the west lot line thereof.

The applicant's request is to allow the front entrance Canopy/Trellis structure to encroach at varying front setbacks; from 26.05' at the curve of the north front property line to 33.27' from the south front property line. Instead of the 35 foot Setback outlined in the code.

The Building Regulation Advisory Board met September 10, 2019 and recommended approval of the variance request, the motion passed with a Board vote of 2 – 1

Melinda Almonte, 395 Golden Beach Drive, GB, FL. spoke in opposition to this request.
Ibrahim Galsky, owner of the property spoke on his own behalf.

Attachments:

- Resolution
- Michael Miller Planning Critique
- Notice of Hearing
- Building Regulation Advisory Board Application
- Copy of resident notification listing
- Summary minutes

Financial Impact: None

TOWN OF GOLDEN BEACH
COMMUNITY DEVELOPMENT
MEMORANDUM

To: **Building Regulatory Advisory Board**
Town of Golden Beach

From: **Michael J. Miller, AICP**
Consultant Town Planner

Date: **August 6th, 2019**

Subject: **Zoning Variance Application**
399 Golden Beach Drive
Front Setback for Architectural Projection
North 19.50' of Lot 39 & All of Lot 40, Block 4, Section E
MMPA Acct. No.: 04-0101-0519

ISSUE

The applicant, John Wiggins as agent for the property owner Galask Construction Enterprises 2, LLC, has submitted an application with the Town for a zoning variance to allow an accessory architectural feature (so-called "Trellis") to be constructed in a portion of the front yard setback of Golden Beach Drive. Because the site is located at the intersection of Golden Beach Drive and The Strand, the wide arc of the right-of-way creates an oddly shaped lot near the corner, which affects the setbacks. The proposed architectural feature is at the northwest corner of the home near the corner arc and is proposed at 26.05' from the arc (nearest portion of structure). The BRAB approved this new single-family home in February 2017 which is now under construction. In January 2018 the developer received approval from the BRAB for a small reflecting pond at the northwest corner of the house (not shown on original house plans). The new proposed architectural feature is planned to cover the reflecting pond and covered entrance path to the front door and extends into the front yard setback. The structure will be anchored to the concrete tie beams with one column to the ground. The proposed structure is shown to include ¾" laminated tempered glass attached to a portion of its top.

The following is the applicant's request:

- Variance from Sec.66-69.2(f) to vary from the required 35' front setback to 26.05' northern portion and 33.27' to the southern portion of the roof of the structure. The roof of the structure will overhang about 3' from the face of the home into the setback; however, the home is placed 36'-3" from the Golden Beach Drive right-of-way.

ANALYSIS

As per the Town's request MMPA has completed our review of the above referenced variance application request and provide the following comments for consideration:

NON-USE VARIANCE JUSTIFICATION

1) *The variance requested is for relief from the provisions of the Town Code.*

   The applicant seeks relief from Sec. 66-69.2 (f) of the Town's Code of Ordinances which is related to the front setback of buildings in Zone 2 (35' from Golden Beach Drive).

2) *In order to recommend the granting of the variance, it must meet all the following criteria:*

   a. *The Variance is in fact a Variance from a zoning regulation as forth within the Zoning Chapter of the Town Code:*

      The applicant references Sec. 66.69.2 - should be Sec.66.69.2(f) Front Setback.

   b. *Special conditions and circumstances exist which are peculiar to the land or structure involved, and which are not applicable to other lands or structures in the same district.*

      The applicant states: "Residential is requiring a 35' setback (should state "front" setback). This property is located at the Golden Beach Drive and The Strand. The street corner reduces the setback diminishes at the front property.

      The shape of the lot due to the corner arc does have an impact concerning the 35' front setback for the proposed architectural feature as requested by the applicant. The lot is not a standard rectangular shape as most lots are.

   c. *The special conditions and circumstances do not result from the actions of the applicant.*

      The applicant's response "Correct".

      The architectural feature is a design choice made by the applicant. The shape of the property is not a standard rectangular lot. The large curve of the platted lot at Golden Beach Drive and The Stand limits the size of the architectural feature which falls with the required 35' front setback. Without the requested front variance setback, the proposed architectural feature must be smaller, but it still could function with a ¾" laminated tempered glass attached to the top.

   d. *Granting the variance requested will not confer on the applicant any special privilege that is denied by the Zoning Chapter of the Town Code to other lands or structures in the same district.*

The applicant's response "Correct".

Approval of the applicant's request would grant them the ability to construct the architectural feature. A few other homes within Town do have similar architectural features in the front yard which provides covered entrance to the home front door. Because the site is located at the intersection of Golden Beach Drive and The Strand the wide arc of the right-of-way creates an oddly shaped lot near the corner, which affects the setbacks.

3) *Literal interpretations of the provisions of the Zoning Chapter of the Town Code would deprive the applicant of rights commonly enjoyed by other properties in the same district under the terms of zoning regulations and would work unnecessary and undue hardship on the applicant.*

The applicant's response "Correct".

Without the variance the applicant could not construct the architectural feature as shown in the proposed site plan submitted with the variance application that meets the Town's Code front setback. The proposed architectural feature could be constructed to meet the Town front setback requirement of 35' but would be smaller and not functional as designed.

4) *The variance granted is the minimum variance that will make possible the reasonable use of the land or structure.*

The applicant's response" Correct".

The variance of 8.95' & 1.72' for the architectural feature a deviation in the front setback would be a reasonable use the grant for the property because the unusual shape of the lot.

5) *The granting of the variance will be in harmony with the general intent and purpose of the Town Code and the variance will not be injurious to the area involved or otherwise detrimental to the public welfare.*

Applicant's response "Correct".

One of the intents of Zoning Codes and other Land Development Regulations (LDRs) is to balance the interest of the general welfare of the Town and that of individual property owners. In this instance the requested variance for the architectural feature could be in harmony with the general intent and purpose of the Town Code and the variance will not be injurious to the area involved or otherwise detrimental to the public welfare.

SUMMARY

MMPA was requested to review and comment of the requested variance related to a proposed architectural feature projection. Because the site is located at the intersection of Golden Beach Drive and The Strand, the wide arc of the right-of-way creates an oddly shaped lot near the corner which affects the setback. The architectural feature / projection technically violates the front setback by 8.95' and 1.72' at the structure furthest extension. The column supporting the structure is not dimensioned but is set back much further. Portions of the roof extend out further than the column or main house – but do not exceed the Code allowances.

TOWN OF GOLDEN BEACH
NOTICE OF PUBLIC HEARING

The **Building Advisory Board** and the **Town Council** of the Town of Golden Beach will hold a Public hearing on the following proposal:

___X___ Variance Request(s)
___X___ Accessory Structures

House under construction – revision to add front entrance Trellis/Canopy cover.

Relief from Town Code Section 66.69-2 Zone Two, (f) front yard setback, (2) a:
(2) Lots fronting east side of Golden Beach Drive. Lots with Frontage on the east side of Golden Beach Drive shall provide front yard Setbacks as follows:
a. For Full Size Lots, no building or part thereof, including garages, shall be erected nearer than 35 feet from the west lot line thereof.

The applicant's request is to allow the front entrance Canopy/Trellis structure to encroach at varying front setbacks; from 26.05' at the curve of the north front property line to 33.27' from the south front property line. Instead of the 35 foot Setback outlined in the code.

JOB ADDRESS:	399 Golden Beach Drive, Golden Beach, FL. 33160
OWNER ADDRESS:	429 Center Island Drive, Golden Beach, FL. 333160
REQUESTED BY:	Galasky Construction Enterprises 2 LLC
LEGAL DESCRIPTION:	N 19.50' of lot 39, and all of lot 40, Blk 4, GB Sect E, PB 8-122
FOLIO NO.:	19-1235-005-0330

The **BUILDING ADVISORY BOARD** will consider this item:

PLACE: GOLDEN BEACH TOWN HALL
1 GOLDEN BEACH DR., GOLDEN BEACH, FL
DATE: AUGUST 13, 2019 AT 6PM

The **TOWN COUNCIL** will consider this item:

PLACE: GOLDEN BEACH TOWN HALL
1 GOLDEN BEACH DR., GOLDEN BEACH, FL.
AUGUST 20, 2019 at 7pm

If you wish to submit written comments for consideration, they should be submitted to the Office of the Golden Beach Town Manager, prior to the scheduled meeting. If you have any questions regarding the proposed action, you may contact the Building Department at (305) 932-0744

DATED: July 25, 2019

LINDA EPPERSON, DIRECTOR BUIDING & ZONING

PURSUANT TO FLA. STATUTE 286.0105, THE TOWN HEREBY ADVISES THE PUBLIC THAT: IF A PERSON DECIDES TO APPEAL ANY DECISION MADE BY THE COUNCIL, BOARD OR COMMITTEE WITH RESPECT TO ANY MATTER CONSIDERED AT ITS MEETING OR HEARING, HE WILL NEED A RECORD OF THE PROCEEDINGS, AND THAT FOR SUCH PURPOSE, AFFECTED PERSONS MAY NEED TO ENSURE THAT A VERBATIM RECORD OF THE PROCEEDING IS MADE, WHICH RECORD INCLUDES THE TESTIMONY AND EVIDENCE UPON WHICH THE APPEAL IS TO BE BASED. ANY INDIVIDUAL WHO BELIEVES HE OR SHE HAS A DISABILITY WHICH REQUIRES A REASONABLE ACCOMMODATION IN ORDER TO PARTICIPATE FULLY AND EFFECTIVELY IN A MEETING OF THE BUILDING REGULATION BOARD MUST SO NOTIFY THE TOWN CLERK, AT (305) 932—0744 AT LEAST 24 HOURS PRIOR TO THE DATE OF THE MEETING.

TOWN OF GOLDEN BEACH
APPLICATION FOR
BUILDING REGULATION ADVISORY BOARD MEETING/HEARING

Property Location: 399 Golden Beach Dr. Meeting Date: _____
Variance Hearing Dates: Advisory Board_____Town Council_____

APPROVAL FROM THE BUILDING REGULATION ADVISORY BOARD IS REQUIRED FOR:

A. Plans for new residence
B. Plans for addition to or exterior alterations of an existing structure.
C. Additional structures on premises of existing residences
D. Review of landscape plan for new construction, renovation or addition to existing residence.
E. Recommendation to Town Council for the approval or denial of variances and special exceptions.
F. Recommendation to Town Council for interpretation regarding apparent conflicts or inconsistencies in the zoning provisions in Chapters 46 & 66

APPLICATION HEARING PROCESS

Building Approval:
Applicant: submit 8 complete packages for approval: each package shall consist of an application, survey, warranty deed and drawings as required. The plans shall be sized as follows: 7 sets; 11" x 17", 1 **full size** set and 1 CD containing all drawings marked with the address. Separate from the landscaping plan submittal

Landscape Approval:
Applicant submit, separate from the Building application, 8 complete packages for approval, Each package shall consist of an application, existing landscape survey, and drawings as required. The plans shall be sized as follows: 7 sets; 11 x 17, 1 **full size** set and 1 CD containing all drawings separate from the building plan submittal.

Zoning Variance Approval:
Submittals for a zoning variance: submit 8 complete packages for approval: each package shall consist of an application, survey, warranty deed and drawings are required. The plans shall be sized as follows: 15 sets; 11" x 17", 1 **full size** set and 1 CD containing all drawings.

As directed by the Building Official or Building Director.

The Building Regulation Advisory Board (B.R.A.B.) meets at 6:00 P. M. on the second Tuesday of every month. Applications must be submitted 30 days, prior to scheduled meeting, **(not including the day of the meeting)**, by 2:00 P.M. that day to allow for preliminary review and for mailing of a public notice if a variance is requested.

Fees must be paid at time of application submittal.

Any variances required must be heard and approved by the Town Council after the Building Regulation Advisory Board has considered the item. The Variance will be heard by the Town Council, in the following month, (on the third Tuesday), after the Building Regulation Advisory Board's action, at the Town Council's regularly scheduled meeting.

Please see page 5 for required documents.

****NOTICE*****

INCOMPLETE APPLICATIONS WILL NOT BE PROCESSED. PLEASE MAKE SURE THAT YOU, THE APPLICANT, HAVE CHECKED AND MARKED ALL ITEMS ON PAGE 5. THANK YOU.

TOWN OF GOLDEN BEACH
APPLICATION FOR
BUILDING REGULATION ADVISORY BOARD HEARING

1. The application deadline date will be strictly complied with. No application shall be accepted after that date and time.

2. The Building Official and/ or the Building Director will review the application package. If it is determined that the application is incomplete, the item will tabled and not placed on the Agenda for that month.

3. During the three (3) week period from deadline date to the hearing date, the following events shall take place in proper order:

    a. During the first week of submittal, the Building Official, or agent will endeavor to review the application, and complete a comment sheet 15 days prior to the meeting. The critique sheet will specify all deficiencies for correction.
    b. The critique sheet will be faxed, or emailed, to the applicant as soon as the review is completed.
    c. If the deficiencies are substantial the application will be moved to the next scheduled meeting/hearing of the B.R.A.B.
    d. If the deficiencies are minor, the applicant must submit the corrections including the revised paperwork within 5 days of the scheduled meeting/hearing. Corrections not received for a scheduled Agenda item will be deferred to the next B.R.A.B. meeting/hearing date.

4. A Notice of Hearing for variance will be mailed no later than 10 days before the date of meeting, as per Town Code.

5. During the third week all applications with plans and documents shall be properly arranged. One complete copy of the package will be given to the processor and the Friday, prior to the scheduled meeting one set will be delivered to each Board member. The Building Dept shall retain all originals for the records.

6. All applicants shall be made aware that incomplete applications or deficiencies not corrected in time as per these regulations, will not be included on the agenda, and are hereby rejected, and will be returned to the applicant.

7. After the meeting, three (3) copies of the approved items (one full size and 2 ledger) shall be retained by the building department, the applicant must request the two reduced sized sets for inclusion into the building permit package.

TOWN OF GOLDEN BEACH
BUILDING REGULATION ADVISORY BOARD
SCHEDULE OF FEES

Appropriate fee shall be paid at time of application. These fees are not refundable.

Type of request	Fee
1. Residence (new construction).....	$300.00
2. Addition/Remodel of existing structure.....	$225.00
3. Fencing, site walls, driveways, pool decks (charged per each item included in the plans)	$150.00
4. Accessory Building or Structure.....	$150.00
5. Swimming pools.....	$100.00
6. Pool deck.....	$100.00
7. Docks.....	$100.00
8. Boat Lifts.....	$100.00
9. Carports, awnings.....	$100.00
10. Landscape plan review; required for new construction, addition and remodeling project. (submit plans with site plan elevations separate from the building plan approval package)	$300.00
11. Resubmissions, based on original fee paid...	75.0%
12. Zoning Variances and special exceptions, per variance or exception:	
a. First variance/ exception.	$750.00
b. Per additional variance/exception, for the same initial varaince (example: request for a dock, affecting two different codes	
c. When a variance is granted, the property owner, at his expense, shall have the resolution for the variance recorded in the public records of Miami-Dade county, and two (2) certified copies of the recorded resolution shall be submitted to the Town for inclusion into the property records	
d. If the Town Council grants a variance, a building permit must be secured within two years of the approval date or the variance will become null and void	
13. Request to the Board for verification of any section of the Zoning Code, For each Section to be verified.....	$100.00

TOWN OF GOLDEN BEACH
BUILDING REGULATION ADIVSORY BOARD
SCHEDULE OF FEES

14. Application for the legalization of construction built without the approval of the B.R.A.B., when the Board should have approved such construction, will be accessed a fee equal to four (4) times the regular fee applicable to the matter.

15. Special Requests for a meeting, variance, or waiver of plat hearing:

a. For matters that have been heard, but the process had not been completed, i.e., tabled subjects, or unfinished subjects to be continued, the applicant must notify the Building & Zoning Department in writing if they would like the item continued

   If the notification is received by the Department before the deadline for the next B.R.A.B. meeting, there will be no charge for the continuance. If the notification is received after the deadline, and the applicant still wants the matter included in the agenda for the next meeting, there will be a special fee of $200.00

b. If the agenda has already been prepared and the applicant wants the matter to be heard, the request must received in writing to be added to the agenda at the beginning of the meeting with the approval of the Building Official or Building & Zoning Director. There will be a special fee of $200.00

c. When a special meeting or hearing of the B.R.A.B. is requested by an applicant, for either a new matter or continuance of a subject already heard, there will be a special fee for a 2 hour time period of $500.00

   If the time limit is exceeded, an additional fee of ½ of the fee will be accessed for the seconded time period $250.00

TOWN OF GOLDEN BEACH
APPLICATION FOR
BUILDING REGULATION ADVISORY BOARD HEARING

Applicant check here	Complete application, sign, and notarize.	Bldg Dept Use
	If a zoning variance is applied for, the petition for variance, pages 9, 10 and 11 shall be submitted with the application and shall include: a. Property Legal Description b. Property Folio number c. Street address d. Owners of record e. Owner and agent names and signatures properly notarized.	
	Eight (8) property surveys, building plans, Warranty Deeds, and applications (1 original, 7 copies). Survey not older than six (6) months. Completed sets are to be submitted as follows: Seven (7) copies no larger than 11" x 17" & 1 original at full size. Sixteen (16) copies are required for a variance, (15 copies no larger than 11" x 17" and 1 original at full size). Each completed package shall consist of 1 each of an application, survey, Warranty Deed and building plans. Submit 1 CD with all documents included	
	Conceptual construction drawings prepared and signed by a licensed design professional that shall include, at a minimum, the following: a. Site plan at a scale not less than 1/8" = 1'-0" (Include grade elevations) b. Proposed Floor Plan views, at a scale not less than ¼"=1'-0" c. Cross and longitudinal sections preferably through vaulted areas, if any. d. Typical exterior wall cross section. e. Full elevations showing flat roof and roof ridge height and any other higher projections. f. Sample board of construction materials to be used. g. Existing and proposed ground floor elevations (NGVD). h. Grading & Drainage Calculations i. Zone 3 Properties: Affidavit of Seawall Conformity	
	Landscaping Plan, separate from building plan package, prepared and signed by a licensed landscape design professional: Each completed package shall consist of 1 each of an application, existing landscape survey, Warranty Deed, landscape plans with building site plans. Seven (7) 11 x 17 and One (1) full size set. Submit 1 CD with all documents included. Mark CD accordingly (separate from building)	
	First Floor and Second Floor area calculations marking the geometrical areas used to calculate the overall floor areas.	
	Colored rendering showing new or proposed addition Work marked with the applicable address.	
	Estimated cost of proposed work. For additions/remodels fair market value of property showing land value and structure value separately.	
	Site plan detailing construction site personnel parking.	

TOWN OF GOLDEN BEACH
APPLICATION FOR
BUILDING REGULATION ADVISORY BOARD HEARING

Application fee:_____

Request hearing in reference to:

New residence/addition: Overhead Trellis (Front) Variance(s): Zone 2- Front Setback
Exterior alterations:_____ Other Structure:_____
Date application filed:_____ For hearing date: 7/9/19

1. Project information:
   Project description: Install a overhead trellis at entrance of residence
   Legal Description: Golden Beach Section E. PB 8-122, N 19.00 of Lot 39 + All lot 40, Blk 4
   Folio #: 19-1235-0330
   Address of Property: 399 Golden Beach Dr Golden Beach, FL 33160

2. Is a variance(s) required: Yes ✓ No_____ How Many? 1
   (If yes, please submit variance application form for each request).

Owner's Name: Grlosky Construction Phone (305) 989-1825 Fax _____
Owner's address: 296 S. Parkway City/State Golden Beach, FL Zip 33160
Email address:_____
Agent: CBJ Construction Inc. Phone (954) 391-7926 Fax _____
Agent's address: 7 NW 3.1 Ave City/State Dania Beach, FL Zip 33004
Email address: jeremy@cbjconstructioninc.com
Architect: _____ Phone _____ Fax _____
Email address:_____
Contractor: _____ Phone _____ Fax _____

3. Describe project and/ or reason for hearing request: As requested by Building department

4. The following information is submitted for assisting in review:

   Building Plans:

   Conceptual: _____ Preliminary: _____ Final: ✓
   Other: _____

5. Estimated cost of work: $ 36,000
   Estimated market value of: Land $ 1,352,981
   Building $ _____
   (Note: If estimated cost of work is 40% of the market value of the building an independent appraisal is required).

TOWN OF GOLDEN BEACH
APPLICATION FOR
BUILDING REGULATION ADVISORY BOARD HEARING

6. Is hearing being requested as a result of a Notice of Violation? No

7. Are there any structures on the property that will be demolished? No

8. Does legal description conform to plat? _____

9. Owner Certification: I hereby certify that I am the owner of record (*) of the property described in this application and that all information supplied herein is true and correct to the best of my knowledge.

Signature of owner(s): _____

Acknowledged before me this May 28 day of, 20 19

Type of identification: _____

ERIN McGOVERN
MY COMMISSION # GG254808
EXPIRES: September 25, 2022

_____
Notary Public

Owner/Power of Attorney Affidavit:

I, being duly sworn, depose and say I am the owner (*) of the property described in this application and that I am aware of the nature and request for: _____ relative to my property and I am hereby authorizing _____ to be my legal representative before the Building Regulation Advisory Board and Town Council.

_____
Signature of owner(s)

Acknowledged before me this _____ day _____ 20_____

Type of identification:

_____ _____
Notary Public

(*) If owner of record is a corporation then the president with corporate seal, the president and the secretary (without corporate seal), or duly authorized agent for the corporation may execute the application, proof that the corporation is a corporation in good standing.

TOWN OF GOLDEN BEACH
APPLICATION FOR
BUILDING REGULATION ADVISORY BOARD HEARING

Property Address: 399 Golden Beach Dr. Golden Beach FL 33160
Legal Description: Golden Beach Section E, PB 8-122, N 19.50' of Lot 39 + all Lot 40, Blk 4
Owner's Name: Galazzy Construction Ent. Phone (954)888-1825 Fax _____
Agent's Name: CBJ Construction Inc Phone (954)391-7926 Fax _____
Board Meeting of: _____

NOTE: 1. **Incomplete applications will not be processed.**
2. Applicant and/or architect must be present at meeting.

Application for: Overhead trellis, front of residence
Lot size: 82 x 150
Lot area: 12,300
Frontage: 82'
Construction Zone: Sea Zone 2
Front setback: 35
Side setback: 20
Rear setback: 35
Coastal Construction: Yes ____ No X East of coastal const. control line: Yes ____ No X
State Road A1A frontage: _____
Swimming pool: X Yes ____ No Existing: X Proposed: _____
Fence Type: 4'4" Aluminum Rail Existing: X Proposed: _____
Finished Floor elevation N.G.V.D.: 6.50'
Seawall: N/A Existing: _____ Proposed: _____
Lot Drainage: Existing
How will rainwater be disposed of on site? Existing swales
_____
Adjacent use (s): _____
Impervious area: 7332
% of impervious area: 57%
Existing ground floor livable area square footage: 3,163
Proposed ground floor livable area square footage: N/A Existing
Existing 2nd floor livable area square footage: N/A Existing
Proposed 2nd floor livable area square footage: N/A Existing
Proposed % of 2nd floor over ground floor: N/A Existing
Vaulted area square footage: N/A Existing
Vaulted height: N/A Existing
Color of main structure: N/A Existing
Color of trim: N/A Existing
Color & material of roof: N/A Existing
Building height (above finished floor elevation): N/A Existing
Swale: (Mandatory 10'-0" from edge of payment, 10 ft. wide x 1 ft. deep minimum):
_____ Existing
Existing trees in Lot: No change in Swale: _____
Proposed trees in Lot: No change in Swale: _____
Number & type of shrubs: _____
Garage Type: _____ Existing: _____ Proposed: _____
Driveway width & type: _____

Signature of Applicant: _____ Date: 5/28/19

TOWN OF GOLDEN BEACH
ACKNOWLEDGEMENT and AFFIDAVIT BY OWNER
Chapter 46 Waterways of the Code of Ordinances
Article IV Seawalls and Docks.

Affidavit by Owner:

Folio No.: 19-1235-005-0330 Address: 397 Golden Beach Dr. Golden Beach, FL 33160

Legal Description: Golden Beach Section E, PB 8-112, N 19.50 ft of Lot 39-all of Lot 40, Blk 4

Being duly sworn, deposes and says: That He/She is the Owner named in the permit application for construction or other related work to be performed on, or in connection with, the premises, as indicated above, and is in agreement that granting of a permit for construction on said premises, agrees to repair, or replace said seawall in question, to a conforming 4 foot elevation and to replace/and or repair any deteriorated seawall or portion thereof, as required by the Town's Code of Ordinances, Article IV "Seawalls and Docks, The Dept. of Environmental Resource Management, and the Florida Building Code 2010.

Signature of Owner or Legal Representative
Print Name:

Sworn to and subscribed before me this May 28 day of, 2019

ERIN McGOVERN
MY COMMISSION # GG254808
EXPIRES September 25, 2022

Notary Public State of Florida at Large

_____Personally know to me _____Produced Identification

Town of Golden Beach
Building Regulatory Advisory Board
Hearing Date

AUG 13 2019

TOWN OF GOLDEN BEACH
BUILDING REGULATION ADVISORY BOARD
APPLICATION FOR
PETITION FOR VARIANCE

APPROVED
DISAPPROVED
VARIANCE REQ:

Date: July 17, 2019

Fee: _____

I John Wiggins hereby petition the Town of Golden Beach for a variance from the terms of the Town of Golden Beach Code of Ordinances affecting property located at:399 Golden Beach Drive, Golden Beach __Folio No19-1235-005-0330

As specified in the attached "Application for Building Regulation Advisory Board" and related supporting material.

1. The Variance requested is for relief from the provisions of **(list section number(s) of the Town of Golden Beach Code of Ordinances): Section 66.69.2 Zone 2 Front Set Back**

2. In order to recommend the granting of the variance, it must meet all the following criteria (please provide a response to each item):

   a. The Variance is in fact a Variance from a zoning regulation as set forth within the Zoning Chapter of the Town Code. Section 66.69.2 Zone 2 Front Set Back

   b. Special conditions and circumstances exist which are peculiar to the land or structure involved, and which are not applicable to other lands or structures in the same district. Residence is requiring a 35" Stet back. This property is located at Golden Beach Drive and the Strand. The street corner reduces the setback diminishes at the front property

   c. The special conditions and circumstances do not result from the actions of the applicant. Correct

   d. Granting the Variance requested will not confer on the applicant any special privilege that is denied by the Zoning Chapter of the Town Code to other lands or structures in the same district. Correct

TOWN OF GOLDEN BEACH
BUILDING REGULATION ADVISORY BOARD
PETITION FOR VARIANCE

3. Literal interpretations of the provisions of the Zoning Chapter of the Town Code would deprive the applicant of rights commonly enjoyed by other properties in the same district under the terms of zoning regulations and would work unnecessary and undue hardship on the applicant. Correct

4. The Variance granted is the minimum Variance that will make possible the reasonable use of the land or structure. Correct

5. The granting of the Variance **will** be in harmony with the general intent and purpose of the Town Code and the Variance will not be injurious to the area involved or otherwise detrimental to the public welfare. Correct

Does the Variance being requested comply with all the above listed criteria? X Yes _____ No

6. Our code states that submission of a written statement is invited and encouraged. Has the applicant (petitioner) explained the variance to the owners of the nearest adjacent residences and sought their approval in writing? ___ Yes ___ No.
Please attach any written letters of no objection to this petition. The requested variance is for the street side and no adjacent neighbor's

7. Is this request related to new construction? X Yes No

8. Is construction in progress? Yes

9. Is this request as a result of a code violation? No

10. Did this condition exist at the time property was acquired? X Yes _____ No

11. Is this request sought as a remedy to a case to be heard, or action taken by the Special Magistrate? No

12. Do you have a building permit? X Yes _____ No

Building Permit No. B-18-12-7903 Date issued: 01/02/2018

TOWN OF GOLDEN BEACH
BUILDING REGULATION ADVISORY BOARD
PETITION FOR VARIANCE
AFFIDAVIT BY OWNER

Affidavit by Owner for Variance Request(s):

Folio No.:19-1235-005-0330 Address: 399 Golden Beach Drive, Golden Beach

Legal Description: Golden Beach Sec E, PV8-122, N19.50FT of Lot 39 & All Lot 40, BLK 4, Lot Size 82.000 X 150

Being duly sworn, deposes and says: That He/She is the Owner named in the application for Building Advisory Board for the hearing dale of_____relating to Variance requests for construction or other work to be performed on, or in connection with, the premises located as indicated in the application.

I acknowledge notification by The Town of Golden Beach, that granting of a variance(s) by The Town Council, is conditioned on the following:

1. That a Building Permit for the contemplated work pursuant to the Variance must be issued within two (2) years from the date of the approval of the Resolution granting such variance request.
2. If a Building Permit is not issued within the two (2) year time limit set then the Resolution granting the Variance approval will be null and void.
3. That as the applicant, and at my own expense, I shall record a certified copy of the Resolution in the public records of Miami-Dade County and return two (2) certified copies to the Golden Beach Town Hall for inclusion into my property records.

Signature of Owner or Legal Representative

Sworn to and subscribed before me this 17 day of, 2019

MELISSA A. RUEDA
MY COMMISSION # GG 252026
EXPIRES: August 23, 2022
Bonded Thru Notary Public Underwriters

Notary Public State of Florida at Large

___ .Personally know to me ___ Produced Identification

2019 FLORIDA LIMITED LIABILITY COMPANY ANNUAL REPORT

DOCUMENT# L14000027310

FILED
Mar 31, 2019
Secretary of State
7871248020CC

Entity Name: GALSKY CONSTRUCTION ENTERPRISES 2, LLC

Current Principal Place of Business:

301 W. HALLANDALE BEACH BLVD.
HALLANDALE BEACH, FL 33009

Current Mailing Address:

301 W. HALLANDALE BEACH BLVD.
HALLANDALE BEACH, FL 33009

FEI Number: NOT APPLICABLE

Certificate of Status Desired: No

Name and Address of Current Registered Agent:

HOWARD B. NADEL, P.A.
301 W. HALLANDALE BEACH BLVD.
HALLANDALE BEACH, FL 33009 US

The above named entity submits this statement for the purpose of changing its registered office or registered agent, or both, in the State of Florida.

SIGNATURE:

Electronic Signature of Registered Agent — Date

Authorized Person(s) Detail :

Title	MGR
Name	GALSKY, ALBERTO
Address	429 CENTER ISLAND
City-State-Zip	GOLDEN BEACH FL 33160

I hereby certify that the information indicated on this report or supplemental report is true and accurate and that my electronic signature shall have the same legal effect as if made under oath; that I am a managing member or manager of the limited liability company or the receiver or trustee empowered to execute this report as required by Chapter 605, Florida Statutes; and that my name appears above, or on an attachment with all other like empowered.

SIGNATURE: ALBERTO GALSKY MGR 03/31/2019

Electronic Signature of Signing Authorized Person(s) Detail — Date

JOHN IBARRA & ASSOCIATES, INC.
Professional Land Surveyors & Mappers
WWW.IBARRALANDSURVEYORS.COM
777 N.W. 72nd AVENUE
SUITE 3028
MIAMI, FLORIDA 33126
PH: (305) 262-0400
FAX: (305) 262-0401

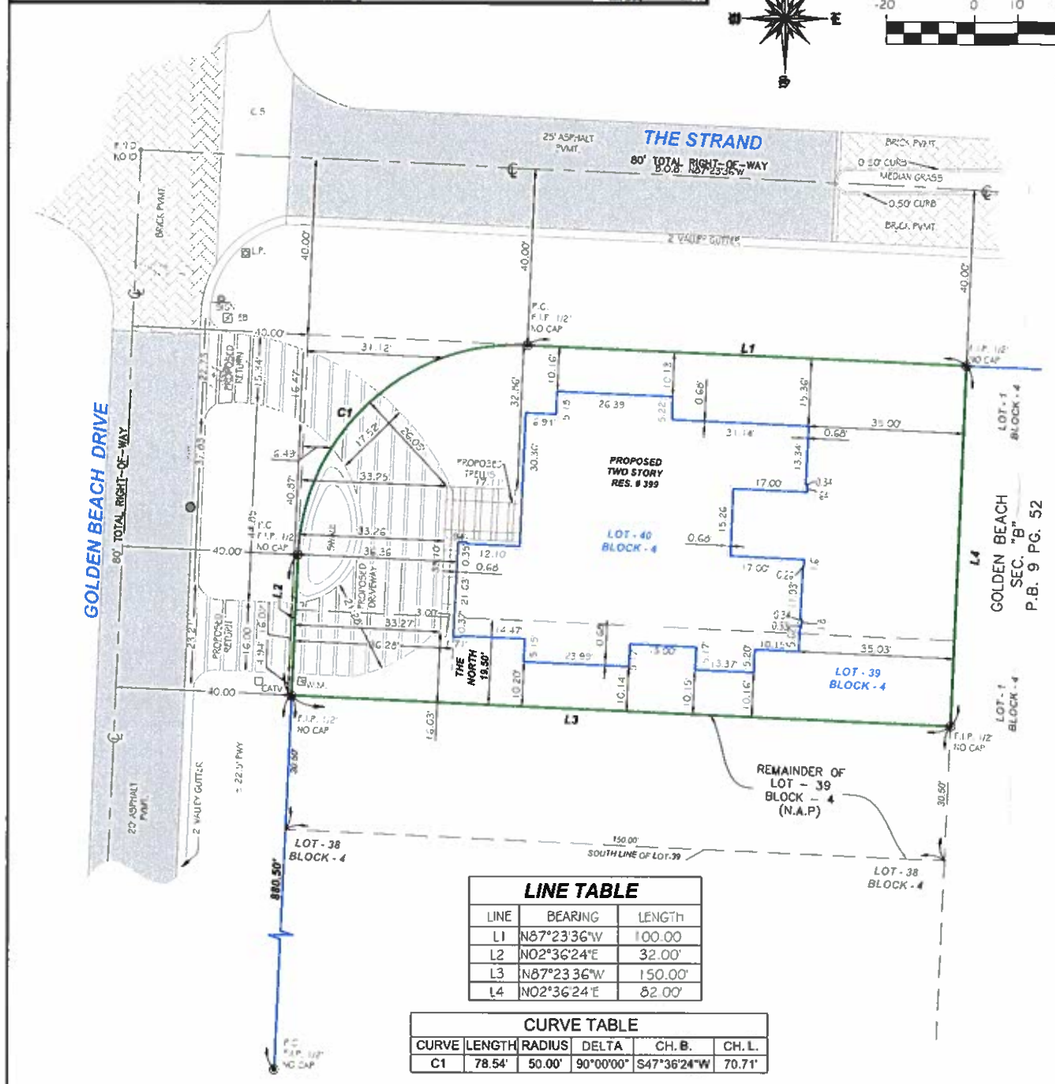
4851 TAMIAMI TRAIL NORTH
SUITE # 200
NAPLES, FL 34103
PH: (239) 540-2660
FAX: (239) 540-2664

SITE PLAN

FOR PROPOSED DRIVEWAY AND TRELLIS

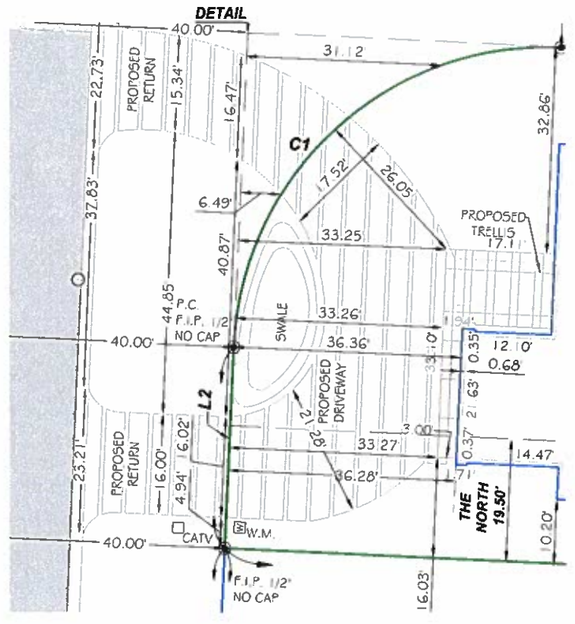
GRAPHIC SCALE
-20 0 10 20 40 80
(IN FEET)
1 INCH = 20 FEET

LOCATION SKETCH
SCALE: N.T.S.



LINE TABLE		
LINE	BEARING	LENGTH
L1	N87°23'36"W	100.00
L2	N02°36'24"E	32.00'
L3	N87°23'36"W	150.00'
L4	N02°36'24"E	82.00'

CURVE TABLE					
CURVE	LENGTH	RADIUS	DELTA	CH. B.	CH. L.
C1	78.54'	50.00'	90°00'00"	S47°36'24"W	70.71'



LEGAL DESCRIPTION:
THE NORTH 19.50' OF LOT 39 AND ALL OF LOT 40, BLOCK 4, SECTION E OF GOLDEN BEACH, ACCORDING TO THE PLAT THEREOF AS RECORDED IN PLAT BOOK 8, AT PAGE 122, OF THE PUBLIC RECORDS OF MIAMI-DADE COUNTY, FLORIDA

PROPERTY ADDRESS:
399 GOLDEN BEACH DRIVE
GOLDEN BEACH, FL 33160

LEGAL NOTES TO ACCOMPANY SKETCH OF SURVEY:
- THERE MAY BE EASEMENTS RECORDED IN THE PUBLIC RECORDS NOT SHOWN ON THIS SURVEY
- EXAMINATIONS OF THE ABSTRACT OF TITLE WILL HAVE TO BE MADE TO DETERMINE RECORDED INSTRUMENTS, IF ANY, AFFECTING THE PROPERTY
- THIS SURVEY IS SUBJECT TO DEDICATIONS, LIMITATIONS, RESTRICTIONS, RESERVATIONS OR EASEMENTS OF RECORD.
- LEGAL DESCRIPTIONS PROVIDED BY CLIENT OR ATTESTING TITLE COMPANY
- BOUNDARY SURVEY MEANS A DRAWING AND/OR A GRAPHIC REPRESENTATION OF THE SURVEY WORK PERFORMED IN THE FIELD. COULD BE DRAWN AT A SHOWN SCALE AND/OR NOT TO SCALE. THE WALLS OR FENCES MAY BE EXAGGERATED FOR CLARITY PURPOSES.
- EASEMENTS AS SHOWN ARE PER PLAT BOOK, UNLESS DEPICTED OTHERWISE
- THE TERM "ENCROACHMENT" MEANS VISIBLE AND ABOVE GROUND ENCROACHMENTS
- ARCHITECTS SHALL VERIFY ZONING REGULATIONS, RESTRICTIONS, SETBACKS AND WILL BE RESPONSIBLE FOR SUBMITTING PLOT PLANS WITH CORRECT INFORMATION FOR "APPROVAL FOR AUTHORIZATION" TO THE PROPER AUTHORITIES IN NEW CONSTRUCTION
- UNLESS OTHERWISE NOTED, THIS FIRM HAS NOT ATTEMPTED TO LOCATE FOOTING AND/OR FOUNDATIONS.
- FENCE OWNERSHIP NOT DETERMINED.
- THIS PLAN OF SURVEY HAS BEEN PREPARED FOR THE EXCLUSIVE USE OF THE ENTITIES NAMED HEREON. THE CERTIFICATE DOES NOT EXTEND TO ANY UNNAMED PARTY

FLOOD ZONE INFORMATION:
THE NFIP FLOOD MAPS HAVE DESIGNATED THE HEREIN DESCRIBED LAND TO BE SITUATED IN

FLOOD ZONE:	"AE"
BASE FLOOD ELEVATION:	6 F.T.
COMMUNITY:	120676
PANEL:	0153
SUFFIX:	L
DATE OF FIRM:	09/11/2009

THE SUBJECT PROPERTY DOES LIE IN A SPECIAL FLOOD HAZARD AREA.

SURVEYOR'S NOTES:
1. IF SHOWN, BEARINGS ARE REFERRED TO AN ASSUMED MERIDIAN, BY SAID PLAT IN THE DESCRIPTION OF THE PROPERTY. IF NOT, BEARINGS ARE THEN REFERRED TO COUNTY, TOWNSHIP MAPS
2. THE CLOSURE IN THE BOUNDARY SURVEY IS ABOVE 1:7500 FT.
3. CERTIFICATE OF AUTHORIZATION LB # 7806

SURVEYOR'S CERTIFICATION:
I HEREBY CERTIFY THAT THIS BOUNDARY SURVEY OF THE PROPERTY DESCRIBED HEREON HAS BEEN PREPARED AND DRAWN UNDER MY SUPERVISION AND COMPLIES WITH THE STANDARDS OF PRACTICE AS SET FORTH BY THE FLORIDA BOARD OF PROFESSIONAL LAND SURVEYORS IN CHAPTER 5J-17.051, F.A.C. PURSUANT TO SECTION 472.027, FLORIDA STATUTES.

07/17/2019
BY: JOHN IBARRA (DATE OF FIELD WORK)
PROFESSIONAL LAND SURVEYOR NO. 5264 STATE OF FLORIDA
NOT VALID WITHOUT THE SIGNATURE AND THE ORIGINAL RAISED SEAL OF A FLORIDA LICENSED SURVEYOR AND MAPPER.

REVISED ON: 07-17-2019 SITE PLAN FOR PROPOSED TRELLIS
REVISED ON: 06-25-2019 SITE PLAN FOR PROPOSED DRIVEWAY
REVISED ON: 04-01-2019 FORM BOARD SURVEY
REVISED ON: 09-01-2017 UPDATED WITH CONSTRUCTION ELEVATION
REVISED ON: 05-13-2015 ADDED TREES
REVISED ON: 01-15-2015 UPDATED WITH CONSTRUCTION ELEVATION
REVISED ON: 02-13-2014 ORIGINAL SURVEY

ABBREVIATIONS AND MEANINGS

LEGEND
- = OVERHEAD UTILITY LINES
- = CONCRETE BLOCK WALL
- = CHAIN LINK FENCE
- = IRON FENCE
- = WOOD FENCE
- = BUILDING SETBACK LINE
- = UTILITY EASEMENT
- = LIMITED ACCESS R/W
- = NON-VEHICULAR ACCESS R/W
- ± 0.00 = EXISTING ELEVATIONS

DRAWN BY:	LK
FIELD DATE:	07/17/2019
SURVEY NO:	14-000523-16
SHEET:	1 OF 1

JOHN IBARRA — LICENSED — NO. 5264 — STATE OF FLORIDA — PROFESSIONAL LAND SURVEYOR
L.B.# 7806 SEAL

Marcos and Lea Carmona
410 Ocean Boulevard
Golden Beach, FL. 33160

Eric B and Keryn L. Fishman
390 Golden Beach Drive
Golden Beach, FL. 33160

Victorine Fleischman and Mary Wolf Trs
403 Golden Beach Drive
Golden Beach, FL. 33160

Nildo and Isora Herrera Trs
400 Ocean Boulevard
Golden Beach, FL. 33160

Galasky Construction Ent. 2 LLC
429 Center Island Drive
Golden Beach, FL. 33160

Franklin and Marjorie Ventura
375 Golden Beach Drive
Golden Beach, FL. 31360

410 Golden Beach LLC
410 Golden Beach Drive
Golden Beach, FL. 33160

Volodymyr Symonenko & Oksana Kirpenko
386 Golden Beach Drive
Golden Beach, FL. 33160

Salomon J. Woldenberg
Ivette Mishne Nestel
20155 NE 38th Ct., Atp 2404
Aventura, FL. 33180

Braintree FLA Corp.
398 Golden Beach Drive
Golden Beach, FL. 33160

Robert A & Allison Friedman
387 Golden Beach Drive
Golden Beach, FL. 33160-2225

Alan Wolfson
368 Golden Beach Drive
Golden Beach, FL. 33160

Annette Willis
Annette Willis Revocable Trust
371 Golden Beach Drive
Golden Beach, FL. 33160

Linda G. Brown Trs
Linda Gail Brown Living Trust
370 Ocean Boulevard
Golden Beach, FL. 33160

Zofia and Barbara Walasek
383 Golden Beach Drive
Golden Beach, Fl. 33160

Aida E & Hugo L. Martinez Trs.
380 Ocean Bouelvard
Golden Beach, FL. 33160

Melinda Almonte
395 Golden Beach Drive
Golden Beach, FL. 33160

Rochelle E. Moore Parrino
417 Golden Beach Drive
Golden Beach, FL. 33160

El Legado LLC
400 Golden Beach Drive
Golden Beach, FL. 33160

Isaac and Suzanne C. Mendal Trust
378 Golden Beach Drive
Golden Beach, FL. 33160

Karen Perry Ebstein
416 Ocean Boulevard
Golden Beach, FL. 33160

Mailing List for Variance Request
399 Golden Beach Drive – 8/13/2019

Eastern
Engineering Group

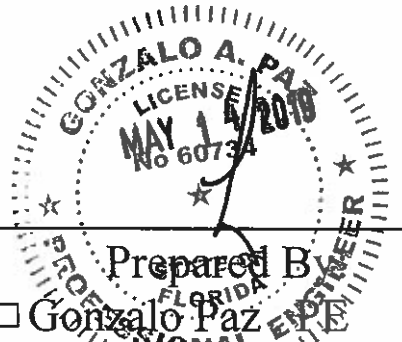
399 GOLDEN BEACH TRELLIS

Job No: 19-0207

MISC. SHOP DRAWINGS

399 Golden beach Dr
Golden Beach, FL 33160-2225

STRUCTURAL CALCULATIONS



Prepared By:
☐ Raissa Lopez, PE
Lic. No. 59399
CAN # 26655

Prepared By:
☐ Gonzalo Paz, PE
Lic. No. 60734
CAN # 26655

3401 NW 82nd Ave, Suite 370, Miami, Florida 33122
Telf. (305) 599-8133 / Email: info@easterneg.com

15

Eastern
Engineering Group

DESIGN CRITERIA:

Calculations based on:

1. 2017 Florida Building Code
2. Minimum Design Loads for Buildings and Other Structures ASCE 7-10
3. Building Code Requirements for Structural Concrete ACI 318-11
4. American Institute of Steel Construction AISC-14ed
5. Aluminum Design Manual 2015
6. Specifications for the Design of Cold-Formed Stainless Steel Structural Members SEI/ASCE8-02

CALCULATION INDEX:

Total Pages= 50

CALCULATION STATEMENT:

To the best of my knowledge, ability, belief and professional judgment, I hereby attest that the manual calculations and computer-generated calculations are in compliance with the existing governing codes.

☐ Prepared By:
Raissa Lopez, PE
Lic. No. 59399
CAN # 26655

☐ Prepared By:
Gonzalo Paz, PE
Lic. No. 60734
CAN # 26655

GONZALO PAZ — LICENSE — STATE OF FLORIDA — REGISTERED PROFESSIONAL ENGINEER
MAY 14 2019

3401 NW 82nd Ave, Suite 370, Miami, Florida 33122
Telf. (305) 599-8133 / Email: info@easterneg.com

Eastern
Engineering Group

WIND ANALYSIS

3401 NW 82nd Ave. Suite 370, Miami, Florida 33122
Telf. (305) 599-8133 / Email: info@easterneg.com

ASCE 7-10 ASD DESIGN=0.6*W

Wind Loads on Buildings (Directional Procedure) per ASCE 7-10

Description: 19-0207 GOLDEN BEACH
Analysis by: M.M.R

User Input Data		
Structure Type	Building	
Basic Wind Speed (V)	175	mph
Struc Category (I, II, III, or IV)	II	
Exposure (B, C, or D)	D	
Struc Nat Frequency (n1)	1	Hz
Slope of Roof	1.0	:12
Slope of Roof (Theta)	4.8	Deg
Type of Roof	Monoslope	
Kd (Directonality Factor)	0.85	
Eave Height (Eht)	26.00	ft
Ridge Height (RHt)	26.00	ft
Mean Roof Height (Ht)	26.00	ft
Width Perp. To Wind Dir (B)	62.00	ft
Width Paral. To Wind Dir (L)	79.00	ft

Calculated Parameters		
Type of Structure		
Height/Least Horizontal Dim	0.42	
Flexible Structure	No	

Calculated Parameters		
Hurricane Prone Region (V>100 mph)		
Table 6-2 Values		
Alpha =	11.500	
zg =	700.000	
At =	0.087	
Bt =	1.070	
Bm =	0.800	
Cc =	0.150	
l =	650.00	ft
Epsilon =	0.125	
Zmin =	7.00	ft

Gust Factor Category I: Rigid Structures - Simplified Method			
Gust1	For rigid structures (Nat Freq > 1 Hz) use 0.85	0.85	
Gust Factor Category II: Rigid Structures - Complete Analysis			
Zm	0.6 * Ht	15.60	ft
Izm	Cc * (33/z)^0.167	0.1700	
Lzm	l*(zm/33)^Epsilon	591.89	ft
Q	(1/(1+0.63*((Min(B,L)+Ht)/Lzm)^0.63))^0.5	0.9169	
Gust2	0.925*((1+1.7*Izm*3.4*Q)/(1+1.7*3.4*Izm))	0.8869	
Gust Factor Summary			
G	Since this is not a flexible structure the lessor of Gust1 or Gust2 are used	0.85	

Fig 26.11-1 Internal Pressure Coefficients for Buildings, Gcpi

Condition	Gcpi	
	Max +	Max -
Open Buildings	0.00	0.00
Partially Enclosed Buildings	0.55	-0.55
Enclosed Buildings	0.18	-0.18
Enclosed Buildings	**0.18**	**-0.18**

ASCE 7-10 ASD DESIGN=0.6*W

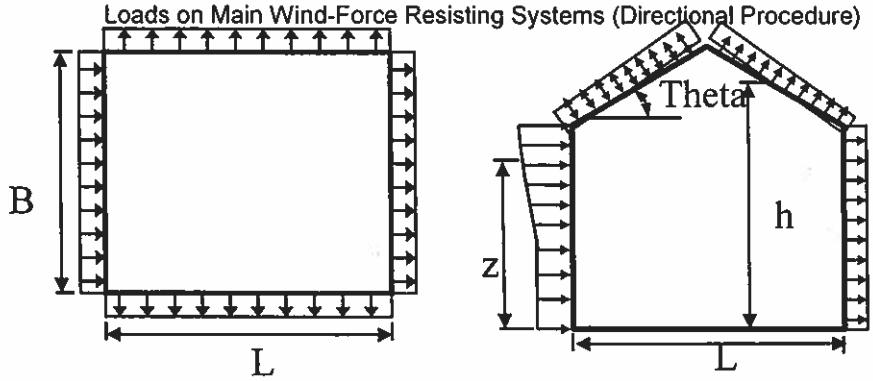
Wind Loads on Buildings (Directional Procedure) per ASCE 7-10

6.5.12.2.1 Design Wind Pressure - Buildings of All Heights

Elev	Kz	Kzt	qz	Pressure (lb/ft^2) Windward Wall*	
0			lb/ft^2	+GCpi	-GCpi
26	1.13	1.00	75.55	22.66	38.98
20	1.08	1.00	72.18	21.29	37.61
15	1.03	1.00	68.65	19.85	36.17

Figure 27.4-1 - External Pressure Coefficients, Cp

Loads on Main Wind-Force Resisting Systems (Directional Procedure)



Variable	Formula	Value	Units
Kh	2.01*(Ht/zg)^(2/Alpha)	1.13	
Kht	Topographic factor (Fig 6-4)	1.00	
Qh	.00256*(V)^2*Kh*Kht*Kd	75.55	psf
Khcc	Comp & Clad: Table 6-3 Case 1	1.13	
Qhcc	.00256*V^2*Khcc*Kht*Kd	75.55	psf

Wall Pressure Coefficients, Cp	
Surface	**Cp**
Windward Wall (See Figure 27.4-1)	0.8

Roof Pressure Coefficients, Cp	
Roof Area (sq. ft.)	-
Reduction Factor	1.00

Calculations for Wind Normal to 62 ft Face *Additional Runs may be req'd for other wind directions*	Cp	Pressure (psf) +GCpi	-GCpi
Leeward Walls (Wind Dir Normal to 62 ft wall)	-0.45	-25.31	-8.99
Leeward Walls (Wind Dir Normal to 79 ft wall)	-0.50	-27.42	-11.11
Side Walls	-0.70	-35.13	-18.81
Overhang Bottom (Applicable on Windward only)	0.80	30.82	30.82
Roof - Wind Normal to Ridge (Theta<10) - for Wind Normal to 62 ft face			
Dist from Windward Edge: 0 ft to 52 ft - Max Cp	-0.18	-15.09	1.22
Dist from Windward Edge: 0 ft to 13 ft - Min Cp	-0.90	-42.83	-26.52
Dist from Windward Edge: 13 ft to 26 ft - Min Cp	-0.90	-42.83	-26.52

ASCE 7-10 ASD DESIGN=0.6*W

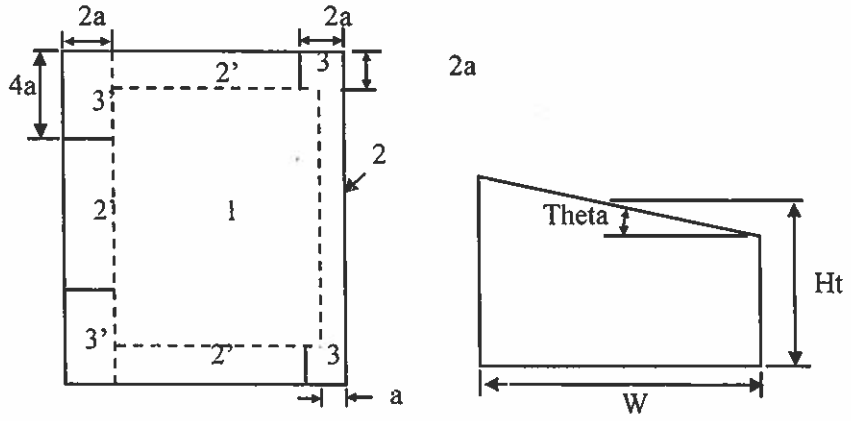
Wind Loads on Buildings (Directional Procedure) per ASCE 7-10

Dist from Windward Edge: 26 ft to 52 ft - Min Cp	-0.50	-45.71	-18.51
Roof - Wind Parallel to Ridge (All Theta) - for Wind Normal to 79 ft face			
Dist from Windward Edge: 0 ft to 52 ft - Max Cp	-0.18	-15.09	1.22
Dist from Windward Edge: 0 ft to 13 ft - Min Cp	-0.90	-42.83	-26.52
Dist from Windward Edge: 13 ft to 26 ft - Min Cp	-0.90	-42.83	-26.52
Dist from Windward Edge: 26 ft to 52 ft - Min Cp	-0.50	-27.42	-11.11
Dist from Windward Edge: > 52 ft	-0.30	-32.86	-5.67

* Horizontal distance from windward edge

Figure 30.4-5 - External Pressure Coefficients, GCp

Loads on Components and Cladding for Buildings w/ Ht <= 60 ft OR 60<Ht<90 & Ht/MIN(B,L)<1
for Monoslope Roofs



a = 6.2 ==> 6.20 ft

Double Click on any data entry line to receive a help Screen

Component	Width (ft)	Span (ft)	Area (ft^2)	Zone	GCp		Wind Press (lb/ft^2)	
					Max	Min	Max	Min
10			10.00	4	0.90	-0.99	48.95	-53.03
10			10.00	5	0.90	-1.26	48.95	-65.27
20			20.00	4	0.85	-0.94	46.79	-50.86
20			20.00	5	0.85	-1.16	46.79	-60.93
50			50.00	4	0.79	-0.88	43.92	-48.00
50			50.00	5	0.79	-1.04	43.92	-55.20
100			100.00	4	0.74	-0.83	41.75	-45.83
100			100.00	5	0.74	-0.94	41.75	-50.86
500			500.00	4	0.63	-0.72	36.72	-40.80
500			500.00	5	0.63	-0.72	36.72	-40.80
10			10.00	1	0.30	-1.10	21.76	-58.02
10			10.00	2	0.30	-1.30	21.76	-67.09
10			10.00	3	0.30	-1.80	21.76	-89.75
20			20.00	1	0.27	-1.10	20.39	-58.02
20			20.00	2	0.27	-1.27	20.39	-65.72
20			20.00	3	0.27	-1.62	20.39	-81.56
50			50.00	1	0.23	-1.10	18.59	-58.02
50			50.00	2	0.23	-1.23	18.59	-63.92

ASCE 7-10 ASD DESIGN=0.6*W

Wind Loads on Buildings (Directional Procedure) per ASCE 7-10

50			50.00	3	0.23	-1.38	18.59	-70.74
100			100.00	1	0.20	-1.10	17.22	-58.02
100			100.00	2	0.20	-1.20	17.22	-62.55
100			100.00	3	0.20	-1.20	17.22	-62.55
10			10.00	1H	0.30	-1.70	13.60	-77.06
10			10.00	2H	0.30	-1.70	13.60	-77.06
10			10.00	3H	0.30	-2.80	13.60	-126.92

Note: * Enter Zone 1, 2, 2', 3, 3' (See sketch), 4 & 5 (Wall Zones calculated per Fig 30.4-1)
* Use 1H, 2H, and 3H for Roof Overhangs (Per Fig 40.4-2A)

Project Name:19-0207 399 Golden Beach Trellis **Date:4/30/2019**

Wind for Solid Freestanding Walls & Signs Design (ε>70%) ASCE 7-10

General Wind Data:

$V := 175.00$ Wind Velocity (mph)

$K_{zt} := 1.00$ Topographic Factor

$K_d := 0.85$ Wind Directionality Factor (see table 26.6-1) ASCE 7-10 ....**Kd=0.85**

$G := 0.85$ Gust Factor (Rigid Structure)

$C_f := 1.85$ Net Force Coefficients (see Figure 6-20 through 6-23)

For Solid Signs: **s/h<0.16 & 0.2<B/s<10.....Cf=1.85**

For Freestanding Walls: **s/h>=1 & B/s=1.....Cf=1.45**

s/h>=1 & B/s=2.....Cf=1.40

s/h>=1 & B/s=5.....Cf=1.35

s/h>=1 & B/s=10.....Cf=1.30

$\alpha := 11.50$

$zg := 700.00$

Values for Terrain exposure constants α and zg:

Exposure B------ Value α =7 , Value zg=1200

Exposure C------ Value α =9.5 , Value zg=900

Exposure D------ Value α =11.5 , Value zg=700

General Sign Data:

$Z := 22.0$ Height of Top of Sign (ft)

$\epsilon := 100.0$ Solidity Ratio of Sign (%)

$$\lambda := 1 - \left(1 - \frac{\epsilon}{100}\right)^{1.5}$$

$\lambda = 1.00$ Solidity Ratio of Sign (%)

Then

$Z := \text{if}(Z < 15, 15, Z)$

$$K_z := 2.01\left(\frac{Z}{zg}\right)^{\frac{2}{\alpha}}$$

$K_z = 1.10$

$q_z := 0.00256 K_z \cdot K_{zt} \cdot K_d \cdot V^2$

$q_z = 73.38$ psf

Allowable Design Wind Loads:

$p_z := 0.6(q_z \cdot G \cdot C_f)$

$p_z = 69.24$ Gross Wind per Actual Solid Area (psf)

$p_{z.EQ} := \max(\lambda \cdot p_z, 10)$

$p_{z.EQ} = 69.24$ Equivalent Wind in Overall Area (psf)

Eastern
Engineering Group

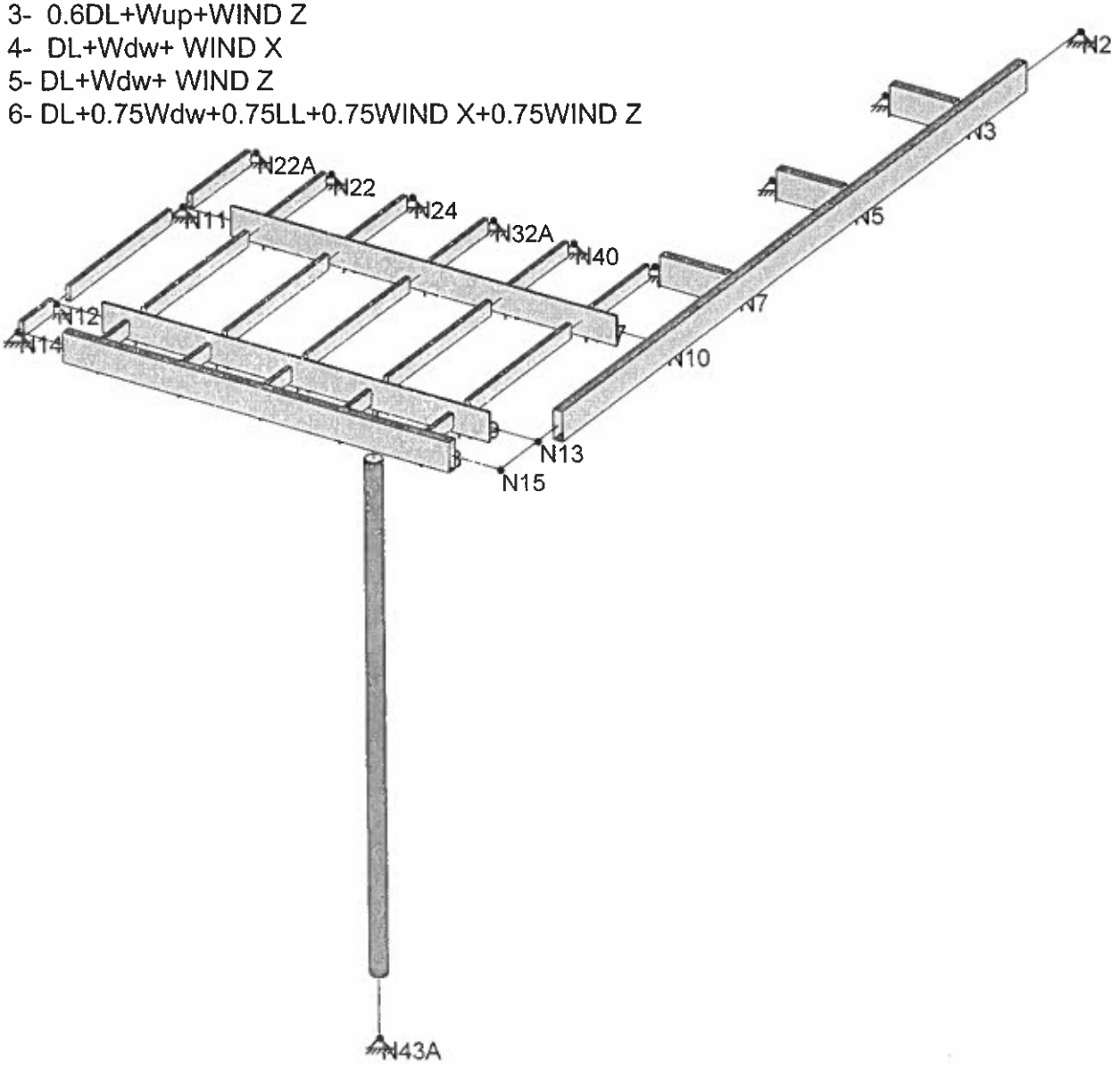
ALUMINUM TRELLIS DESIGN

3401 NW 82nd Ave. Suite 370. Miami, Florida 33122
Telf. (305) 599-8133 / Email: info@easterneg.com

ALUMINUM TRELLIS DESIGN

LOADS:
DL= 10 PSF
LL= 20 PSF
WL= 69.24 PSF
Wup= 77.06 PSF

LOAD COMBINATION
1- DL+LL
2- 0.6DL+Wup+WIND X
3- 0.6DL+Wup+WIND Z
4- DL+Wdw+ WIND X
5- DL+Wdw+ WIND Z
6- DL+0.75Wdw+0.75LL+0.75WIND X+0.75WIND Z



*Solution: Envelope

E.G.G	Aluminum Trellis	SK - 1
M.M.R		Apr 30, 2019 at 11:19 AM
19-0207		Column FLAT BAR R3D

Eastern
Engineering Group
3401 NW 82nd Avenue,
Suite 370, Miami, FL 33122
(305) 599-8133
info@easterneg.com
www.easterneg.com

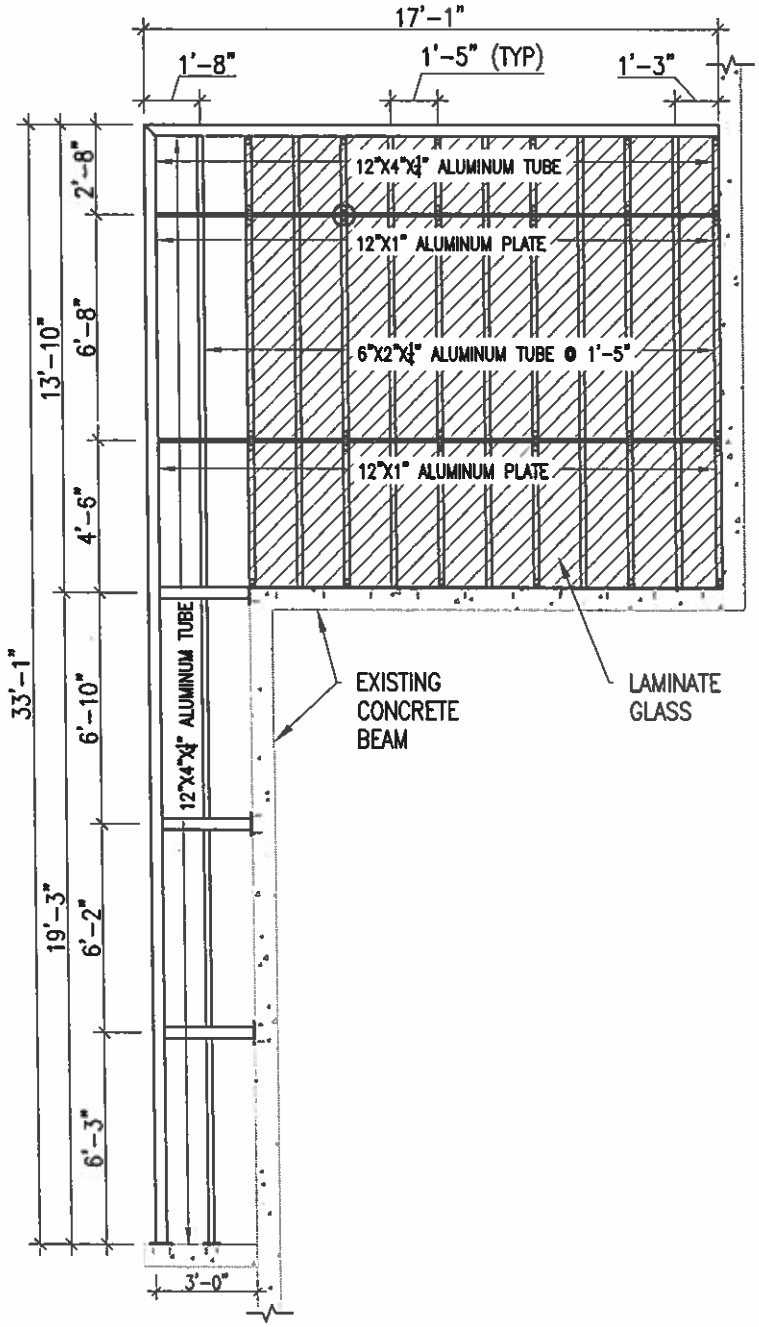
PROJ. NAME: _____

PROJ No.: _____ DATE: _____

DESIGNED BY: _____ PAGE: _____

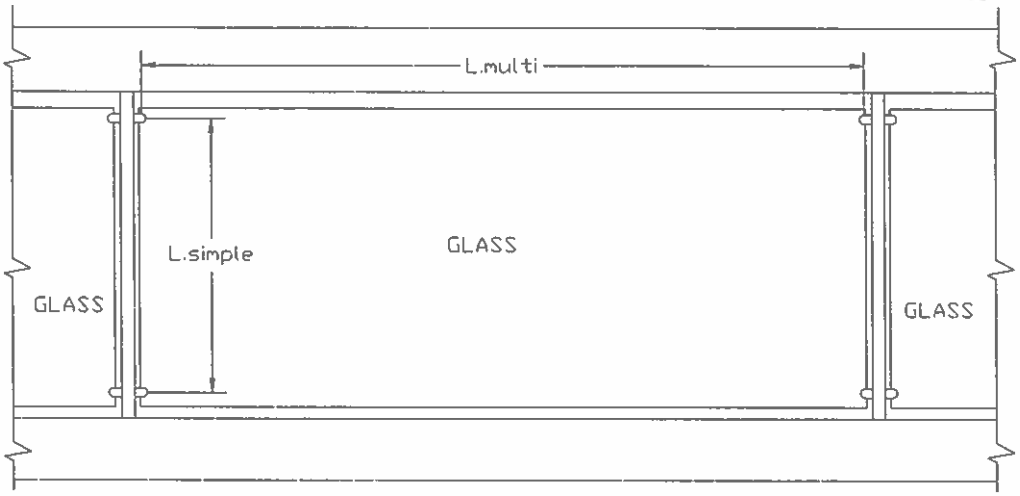
REVIEWED BY: _____

ALUMINUM TRELLIS PLAN VIEW



Project Name:19-0207 399 Golden Beach Trellis **Date:4/30/2019**

Four Side Puntual One Span Supported Glass Design



Loads Data:

$P_{200} := 400.00$ Concentrated Load (lbs)

$q_{wind} := 77.06$ Wind Uniform Load (psf)

Glass Data:

$E := 10400000.00$ Modulus of Elasticity of Glass (psi)

$Mr_{flexure} := 24000.00$ Modulus of Rupture of Glass in Flexure (psi)

$Mr_{shear} := 12000.00$ Modulus of Rigity of glass in Shear (psi)

$SF := 4.00$ Safety Factor

Geometric Glass Railing Data:

$L_{simple} := 36.0$ Vertical Span of Glass Pannel (**Distance Between Support**) (in)

$L_{multi} := 80.0$ Horizontal Span of Glass Pannel (**Distance Between Support**) (in)

$t := 0.75$ Thickness of Glass Pannel (in)

Then

$$F_b := \frac{Mr_{flexure}}{SF}$$ $F_b = 6000.00$ psi

$$F_v := \frac{Mr_{shear}}{SF}$$ $F_v = 3000.00$ psi

$$S_{simple} := \frac{\min(0.5L_{simple}, L_{multi}) \cdot t^2}{6}$$ $S_{simple} = 1.69$ in^3

$$S_{multi} := \frac{\min(0.5L_{multi}, L_{simple}) \cdot t^2}{6}$$ $S_{multi} = 3.38$ in^4

$$A_{simple} := \min(0.5L_{simple}, L_{multi}) \cdot t$$ $A_{simple} = 13.50$ in^2

$$A_{multi} := \min(0.5L_{multi}, L_{simple}) \cdot t$$ $A_{multi} = 27.00$ in^2

Actual Glass Moment:

Concentrated Load = 200 lbs.

$$M_{200.simple} := \frac{P_{200} \cdot L_{simple}}{4}$$ $M_{200.simple} = 3600.00$ lbs – in

$$M_{200.multi} := \frac{P_{200} \cdot L_{multi}}{4}$$ $M_{200.multi} = 8000.00$ lbs – in

Wind Load (psf)

$$M_{wind.simple} := \frac{\left(\frac{q_{wind}}{144}\right) \cdot \min(0.5L_{simple}, L_{multi}) \cdot L_{simple}^2}{8}$$ $M_{wind.simple} = 1560.47$ lbs – in

$$M_{wind.multi} := \frac{\left(\frac{q_{wind}}{144}\right) \cdot \min(0.5L_{multi}, L_{simple}) \cdot L_{multi}^2}{8}$$ $M_{wind.multi} = 15412.00$ lbs – in

$$M_{actual.simple} := \max(M_{200.simple}, M_{wind.simple})$$ $M_{actual.simple} = 3600.00$ lbs – in

$$M_{actual.multi} := \max(M_{200.multi}, M_{wind.multi})$$ $M_{actual.multi} = 15412.00$ lbs – in

Actual Glass Shear:

Concentrated Load = 200 lbs.

$V_{200} := P_{200}$ $\quad V_{200} = 400.00$ lbs

Wind Load (psf)

$$V_{wind.simple} := \frac{\left(\frac{q_{wind}}{144}\right) \cdot \min(0.5L_{simple}, L_{multi}) \cdot L_{simple}}{2}$$ $\quad V_{wind.simple} = 173.38$ lbs

$$V_{wind.multi} := \frac{\left(\frac{q_{wind}}{144}\right) \cdot \min(0.5L_{multi}, L_{simple}) \cdot L_{multi}}{2}$$ $\quad V_{wind.multi} = 770.60$ lbs

$V_{actual.simple} := \max(V_{200}, V_{wind.simple})$ $\quad V_{actual.simple} = 400.00$ lbs

$V_{actual.multi} := \max(V_{200}, V_{wind.multi})$ $\quad V_{actual.multi} = 770.60$ lbs

Section Required:

Bending Design: Section Modulus Required

$$Sx_{r.simple} := \frac{M_{actual.simple}}{F_b}$$ $\quad Sx_{r.simple} = 0.6$ in^3

$$Sx_{r.multi} := \frac{M_{actual.multi}}{F_b}$$ $\quad Sx_{r.multi} = 2.57$ in^3

Shear Design: Area Required

$$A_{r.simple} := \frac{V_{actual.simple}}{F_v}$$ $\quad A_{r.simple} = 0.13$ in^2

$$A_{r.multi} := \frac{V_{actual.multi}}{F_v}$$ $\quad A_{r.multi} = 0.26$ in^2

Section Provided:

$BENDING_{simple} := if(Sx_{r.simple} \geq \min(S_{simple}), "N.G", "OK")$ $\quad BENDING_{simple} = "OK"$

$BENDING_{multi} := if(Sx_{r.multi} \geq \min(S_{multi}), "N.G", "OK")$ $\quad BENDING_{multi} = "OK"$

$SHEAR_{simple} := if(A_{r.simple} \geq A_{simple}, "N.G", "OK")$ $\quad SHEAR_{simple} = "OK"$

$SHEAR_{multi} := if(A_{r.multi} \geq A_{multi}, "N.G", "OK")$ $\quad SHEAR_{multi} = "OK"$

Joint Coordinates and Temperatures

	Label	X [ft]	Y [ft]	Z [ft]	Temp [F]	Detach From Diaphr...
1	N2	-164.926523	0	-29.973898	0	
2	N3	-158.289674	0	-29.973898	0	
3	N4	-158.289674	0	-27.150001	0	
4	N5	-151.819492	0	-29.973898	0	
5	N6	-151.819492	0	-27.150001	0	
6	N7	-145.182643	0	-29.973898	0	
7	N10	-141.015976	0	-29.973898	0	
8	N11	-141.015976	0	-13.057232	0	
9	N22A	-145.182643	0	-13.057	0	
10	N12	-133.682643	0	-13.057232	0	
11	N13	-133.682643	0	-29.973898	0	
12	N14	-131.509857	0	-13.057232	0	
13	N15	-131.509857	0	-29.973898	0	
14	N22	-145.182643	0	-15.723932	0	
15	N30	-141.015976	0	-15.723932	0	
16	N45	-131.509857	0	-15.723932	0	
17	N46	-133.682643	0	-15.723932	0	
18	N24	-145.182643	0	-18.553932	0	
19	N25	-141.015976	0	-18.553932	0	
20	N26	-131.509857	0	-18.553932	0	
21	N27	-133.682643	0	-18.553932	0	
22	N32A	-145.182643	0	-21.383932	0	
23	N33	-141.015976	0	-21.383932	0	
24	N34	-131.509857	0	-21.383932	0	
25	N35	-133.682643	0	-21.383932	0	
26	N40	-145.182643	0	-24.213932	0	
27	N41	-141.015976	0	-24.213932	0	
28	N42	-131.509857	0	-24.213932	0	
29	N43	-133.682643	0	-24.213932	0	
30	N43A	-133.682643	-21.5	-24.213932	0	
31	N36	-145.182643	0	-27.043932	0	
32	N37	-141.015976	0	-27.043932	0	
33	N38	-131.509857	0	-27.043932	0	
34	N39	-133.682643	0	-27.043932	0	

Hot Rolled Steel Properties

	Label	E [ksi]	G [ksi]	Nu	Therm (\1E5 F)	Density[k/ft^3]	Yield[ksi]
1	Aluminum	10100	3800	.33	.1	.169	15.03

Hot Rolled Steel Section Sets

	Label	Shape	Type	Design List	Material	Design Rules	A [in2]	Iyy [in4]	Izz [in4]	J [in4]
1	FRAME	HSS12X4X4	Beam	Tube	Aluminum	Typical	7.085	21.006	118.934	59.706
2	INTER	HSS6X2X4	Beam	Tube	Aluminum	Typical	3.365	2.205	13.093	6.542
3	COLUMN	PIPE 6.0	Column	Pipe	Aluminum	Typical	5.22	26.5	26.5	52.9
4	PLATE	12"X1"	Beam	None	Aluminum	Typical	12	1	144	3.79

Basic Load Cases

	BLC Description	Category	X Gravity	Y Gravity	Z Gravity	Joint	Point	Distribut..	Area (M...	Surface ...
1	DL	DL		-1				18		
2	LL	LL						18		
3	WIND UP	WL						18		
4	WIND DW	WL						18		

Basic Load Cases (Continued)

	BLC Description	Category	X Gravity	Y Gravity	Z Gravity	Joint	Point	Distribut...	Area (M...	Surface ...
5	WIND X	WL						1		
6	WIND Z	WL						1		

Load Combinations

	Description	S...	P...	S...	B...	Fa...	B...	Fa...	BLC	Fa...	B...	Fa...	B...	Fa...	B...	Fa...	B...	Fa...	B...	Fa...
1	DL+LL	Y...	Y		1	1	2	1												
2	0.6DL+Wup+WIND X	Y...	Y		1	.6	3	1	5	1										
3	0.6DL+Wup+WIND Z	Y...	Y		1	.6	3	1	6	1										
4	DL+Wdw+ WIND X	Y...	Y		1	1	4	1	5	1										
5	DL+Wdw+ WIND Z	Y...	Y		1	1	4	1	6	1										
6	DL+0.75Wdw+0.75LL+0.75WIND X+0.75WIND Z	Y...	Y		1	1	4	.75	2	.75	5	.75	6	.75						

Member Distributed Loads (BLC 1 : DL)

	Member Label	Direction	Start Magnitude[k/ft,deg]	End Magnitude[k/ft,...	Start Location[ft,%]	End Location[ft,%]
1	M16	Y	-.02	-.02	0	0
2	M24	Y	-.02	-.02	0	0
3	M32	Y	-.02	-.02	0	0
4	M26	Y	-.02	-.02	0	0
5	M27	Y	-.02	-.02	0	0
6	M28	Y	-.02	-.02	0	0
7	M32B	Y	-.02	-.02	0	0
8	M33A	Y	-.02	-.02	0	0
9	M34A	Y	-.02	-.02	0	0
10	M35A	Y	-.02	-.02	0	0
11	M36	Y	-.02	-.02	0	0
12	M37	Y	-.02	-.02	0	0
13	M38	Y	-.01	-.01	0	0
14	M39	Y	-.01	-.01	0	0
15	M40	Y	-.01	-.01	0	0
16	M31A	Y	-.01	-.01	0	0
17	M32A	Y	-.01	-.01	0	0
18	M33	Y	-.01	-.01	0	0

Member Distributed Loads (BLC 2 : LL)

	Member Label	Direction	Start Magnitude[k/ft,deg]	End Magnitude[k/ft,...	Start Location[ft,%]	End Location[ft,%]
1	M16	Y	-.02	-.02	0	0
2	M24	Y	-.02	-.02	0	0
3	M32	Y	-.02	-.02	0	0
4	M26	Y	-.02	-.02	0	0
5	M27	Y	-.02	-.02	0	0
6	M28	Y	-.02	-.02	0	0
7	M32B	Y	-.02	-.02	0	0
8	M33A	Y	-.02	-.02	0	0
9	M34A	Y	-.02	-.02	0	0
10	M35A	Y	-.02	-.02	0	0
11	M36	Y	-.02	-.02	0	0
12	M37	Y	-.02	-.02	0	0
13	M38	Y	-.01	-.01	0	0
14	M39	Y	-.01	-.01	0	0
15	M40	Y	-.01	-.01	0	0
16	M31A	Y	-.01	-.01	0	0
17	M32A	Y	-.01	-.01	0	0
18	M33	Y	-.01	-.01	0	0

Member Distributed Loads (BLC 3 : WIND UP)

	Member Label	Direction	Start Magnitude[k/ft,deg]	End Magnitude[k/ft,...	Start Location[ft,%]	End Location[ft,%]
1	M16	Y	.156	.156	0	0
2	M24	Y	.156	.156	0	0
3	M32	Y	.156	.156	0	0
4	M26	Y	.156	.156	0	0
5	M27	Y	.156	.156	0	0
6	M28	Y	.156	.156	0	0
7	M32B	Y	.156	.156	0	0
8	M33A	Y	.156	.156	0	0
9	M34A	Y	.156	.156	0	0
10	M35A	Y	.156	.156	0	0
11	M36	Y	.156	.156	0	0
12	M37	Y	.156	.156	0	0
13	M38	Y	.078	.078	0	0
14	M39	Y	.078	.078	0	0
15	M40	Y	.078	.078	0	0
16	M31A	Y	.078	.078	0	0
17	M32A	Y	.078	.078	0	0
18	M33	Y	.078	.078	0	0

Member Distributed Loads (BLC 4 : WIND DW)

	Member Label	Direction	Start Magnitude[k/ft,deg]	End Magnitude[k/ft,...	Start Location[ft,%]	End Location[ft,%]
1	M16	Y	-.028	-.028	0	0
2	M24	Y	-.028	-.028	0	0
3	M32	Y	-.028	-.028	0	0
4	M26	Y	-.028	-.028	0	0
5	M27	Y	-.028	-.028	0	0
6	M28	Y	-.028	-.028	0	0
7	M32B	Y	-.028	-.028	0	0
8	M33A	Y	-.028	-.028	0	0
9	M34A	Y	-.028	-.028	0	0
10	M35A	Y	-.028	-.028	0	0
11	M36	Y	-.028	-.028	0	0
12	M37	Y	-.028	-.028	0	0
13	M38	Y	-.014	-.014	0	0
14	M39	Y	-.014	-.014	0	0
15	M40	Y	-.014	-.014	0	0
16	M31A	Y	-.014	-.014	0	0
17	M32A	Y	-.014	-.014	0	0
18	M33	Y	-.014	-.014	0	0

Member Distributed Loads (BLC 5 : WIND X)

	Member Label	Direction	Start Magnitude[k/ft,deg]	End Magnitude[k/ft,...	Start Location[ft,%]	End Location[ft,%]
1	M38A	X	.046	.046	0	0

Member Distributed Loads (BLC 6 : WIND Z)

	Member Label	Direction	Start Magnitude[k/ft,deg]	End Magnitude[k/ft,...	Start Location[ft,%]	End Location[ft,%]
1	M38A	Z	-.046	-.046	0	0

Envelope Joint Reactions

	Joint		X [k]	LC	Y [k]	LC	Z [k]	LC	MX [k-ft]	LC	MY [k-ft]	LC	MZ [k-ft]	LC
1	N4	max	0	4	.012	4	0	4	0	1	0	1	0	1
2		min	0	6	.007	2	0	5	0	1	0	1	0	1
3	N6	max	0	6	.012	5	0	5	0	1	0	1	0	1
4		min	0	4	.007	3	0	4	0	1	0	1	0	1
5	N11	max	0	4	1.091	6	.004	5	0	1	0	1	0	1

Envelope Joint Reactions (Continued)

	Joint		X [k]	LC	Y [k]	LC	Z [k]	LC	MX [k-ft]	LC	MY [k-ft]	LC	MZ [k-ft]	LC
6		min	0	5	-2.272	3	0	1	0	1	0	1	0	1
7	N12	max	0	4	.27	6	.487	5	0	1	0	1	0	1
8		min	0	3	-.819	2	0	4	0	1	0	1	0	1
9	N14	max	0	1	.207	6	.006	5	0	1	0	1	0	1
10		min	0	4	-.284	2	0	1	0	1	0	1	0	1
11	N22	max	.001	4	.125	6	0	1	0	1	0	1	0	1
12		min	0	1	-.295	3	0	6	0	1	0	1	0	1
13	N2	max	.012	4	.139	6	0	5	0	1	0	1	0	1
14		min	0	1	-.003	3	0	4	0	1	0	1	0	1
15	N22A	max	0	4	.038	6	0	5	0	1	0	1	0	1
16		min	0	5	-.082	3	0	4	0	1	0	1	0	1
17	N24	max	.012	4	.125	6	0	1	0	1	0	1	0	1
18		min	0	1	-.295	3	0	6	0	1	0	1	0	1
19	N32A	max	0	1	.125	6	0	6	0	1	0	1	0	1
20		min	-.052	4	-.296	2	0	1	0	1	0	1	0	1
21	N40	max	0	1	.128	6	0	6	0	1	0	1	0	1
22		min	-.418	4	-.302	2	0	1	0	1	0	1	0	1
23	N36	max	0	1	.653	6	0	1	0	1	0	1	0	1
24		min	-.052	4	-.923	3	-.002	5	0	1	0	1	0	1
25	N43A	max	0	5	.327	6	.496	1	0	1	0	1	0	1
26		min	-.496	2	-3.446	3	0	4	0	1				1
27	Totals:	max	0	3	5.25	6	.992	5						
28		min	-.992	4	-8.991	3	0	1						

Plate Reaction

Beam Reaction

Column Reaction

Envelope Joint Displacements

	Joint		X [in]	LC	Y [in]	LC	Z [in]	LC	X Rotation ...	LC	Y Rotation ...	LC	Z Rotation [r...	LC
1	N2	max	0	1	0	3	0	4	1.05e-4	6	4.056e-8	5	1.934e-3	3
2		min	0	4	0	6	0	5	-3.786e-4	2	0	4	-2.16e-3	6
3	N3	max	0	1	.153	3	0	5	1.05e-4	6	0	4	1.898e-3	3
4		min	0	4	-.164	6	0	4	-3.786e-4	2	-8.809e-8	5	-1.841e-3	6
5	N4	max	0	6	0	2	0	5	4.524e-3	3	4.084e-7	4	1.898e-3	3
6		min	0	4	0	4	0	4	-4.831e-3	6	0	1	-1.841e-3	6
7	N5	max	0	1	.294	3	0	4	1.05e-4	6	3.402e-7	5	1.665e-3	3
8		min	0	4	-.281	6	0	5	-3.786e-4	2	0	4	-1.128e-3	6
9	N6	max	0	4	0	3	0	4	8.668e-3	3	8.065e-7	4	1.665e-3	3
10		min	0	6	0	5	0	5	-8.281e-3	6	0	1	-1.128e-3	6
11	N7	max	0	1	.404	3	0	5	1.05e-4	6	0	4	1.015e-3	3
12		min	0	4	-.338	6	0	4	-3.786e-4	2	-1.423e-6	5	-3.338e-4	6
13	N10	max	0	1	.405	3	0	1	1.05e-4	6	6.039e-6	5	1.094e-3	6
14		min	0	4	-.32	6	0	5	-3.786e-4	2	0	1	-1.146e-3	3
15	N11	max	0	5	0	3	0	1	6.334e-3	3	3.097e-8	4	1.78e-4	2
16		min	0	4	0	6	0	5	-3.555e-3	6	0	5	-7.924e-5	6
17	N22A	max	0	5	0	3	0	4	6.334e-3	3	0	5	3.519e-5	3
18		min	0	4	0	6	0	5	-3.555e-3	6	-1.492e-8	4	-1.728e-5	6
19	N12	max	0	3	0	2	0	4	3.774e-4	2	1.029e-7	4	9.311e-5	6
20		min	0	4	0	6	0	5	2.252e-5	6	0	3	-1.974e-4	3
21	N13	max	0	1	.18	3	0	1	1.05e-4	6	0	1	2.224e-3	6
22		min	0	4	-.149	6	0	5	-3.786e-4	2	-1.173e-5	5	-3.06e-3	3
23	N14	max	0	4	0	2	0	1	1.41e-3	3	0	1	6.283e-5	3
24		min	0	1	0	6	0	5	-1.012e-3	6	-5.173e-8	4	-2.987e-5	6
25	N15	max	0	1	.102	3	0	4	1.05e-4	6	4.72e-7	4	6.283e-5	3
26		min	0	4	-.093	6	0	5	-3.786e-4	2	0	1	-2.987e-5	6
27	N22	max	0	1	0	3	0	6	5.733e-3	3	2.353e-8	5	4.432e-3	3
28		min	0	4	0	6	0	1	-3.273e-3	6	0	1	-2.431e-3	6
29	N30	max	0	1	.198	3	0	1	5.733e-3	3	0	5	1.058e-4	6
30		min	0	4	-.112	6	0	5	-3.273e-3	6	-8.651e-8	4	-3.077e-5	3

Envelope Joint Displacements (Continued)

	Joint		X [in]	LC	Y [in]	LC	Z [in]	LC	X Rotation ...	LC	Y Rotation ...	LC	Z Rotation [r...	LC
31	N45	max	0	1	.045	3	0	4	1.291e-3	3	9.964e-7	4	6.283e-5	3
32		min	0	4	-.032	6	0	5	-9.428e-4	6	0	1	-2.987e-5	6
33	N46	max	0	1	.01	2	0	1	1.884e-4	2	0	5	4.29e-4	6
34		min	0	4	.002	6	0	5	8.33e-5	6	-2.957e-7	4	-6.487e-4	3
35	N24	max	0	1	0	3	0	6	4.046e-3	3	4.85e-8	5	7.817e-3	3
36		min	0	4	0	6	0	1	-2.487e-3	6	0	1	-4.417e-3	6
37	N25	max	0	1	.367	3	0	1	4.046e-3	3	1.876e-6	4	3.021e-4	6
38		min	0	4	-.211	6	0	5	-2.487e-3	6	0	5	-2.523e-4	3
39	N26	max	0	1	.083	3	0	4	9.446e-4	3	4.478e-7	4	6.283e-5	3
40		min	0	4	-.061	6	0	5	-7.336e-4	6	0	1	-2.987e-5	6
41	N27	max	0	1	.011	2	0	1	9.681e-5	6	5.302e-6	4	7.855e-4	6
42		min	0	4	.006	6	0	5	-1.045e-4	3	0	3	-1.128e-3	3
43	N32A	max	0	4	0	2	0	1	1.886e-3	3	7.347e-8	5	9.846e-3	3
44		min	0	1	0	6	0	6	-1.494e-3	6	0	1	-5.774e-3	6
45	N33	max	0	4	.469	3	0	1	1.886e-3	3	0	3	4.985e-4	6
46		min	0	1	-.279	6	0	5	-1.494e-3	6	-1.435e-5	4	-4.737e-4	3
47	N34	max	0	4	.108	3	0	4	5.001e-4	3	0	1	6.283e-5	3
48		min	0	1	-.082	6	0	5	-4.[illegible]		[illegible]-5	4	-2.987e-5	6
49	N35	max	0	4	.007	2	0	1	3.2[illegible]		[illegible]	3	1.142e-3	6
50		min	0	1	.006	6	0	5	-2.066e-4	6	-3.978e-5	4	-1.607e-3	3
51	N40	max	0	4	0	2	0	1	-1.346e-4	3	9.844e-8	5	1.041e-2	3
52		min	0	1	0	6	0	5	-5.907e-4	6	0	1	-6.464e-3	6
53	N41	max	0	4	.497	3	0	1	-1.346e-4	3	0	1	6.948e-4	6
54		min	0	1	-.313	6	0	5	-5.907e-4	6	-2.4e-7	4	-6.952e-4	3
55	N42	max	.002	4	.118	3	0	4	7.62e-5	3	0	1	6.283e-5	3
56		min	0	1	-.093	6	0	5	-1.986e-4	6	-1.327e-8	4	-2.987e-5	6
57	N43	max	.002	4	.021	3	0	1	1.139e-3	3	0	1	1.499e-3	6
58		min	0	1	-.014	6	0	5	-1.106e-3	6	-4.324e-7	4	-2.085e-3	3
59	N43A	max	0	2	0	3	0	4	0	1	0	1	0	1
60		min	0	5	0	6	0	3	-1.03e-2	5	-4.324e-7	4	-1.03e-2	4
61	N36	max	0	4	0	3	0	5	1.143e-2	3	1.083e-6	4	5.691e-3	3
62		min	0	1	0	6	0	1	-9.562e-3	6	0	1	-3.738e-3	6
63	N37	max	0	4	.466	3	0	1	-6.781e-5	6	1.535e-5	4	8.912e-4	6
64		min	0	5	-.322	6	0	5	-1.41e-3	3	0	1	-9.167e-4	3
65	N38	max	0	4	.114	3	0	4	3.904e-6	6	3.355e-5	4	6.283e-5	3
66		min	0	5	-.095	6	0	5	-2.362e-4	2	0	1	-2.987e-5	6
67	N39	max	0	4	.085	3	0	1	2.388e-3	3	4.16e-5	4	1.855e-3	6
68		min	0	5	-.07	6	0	5	-2.027e-3	6	0	1	-2.564e-3	3

Max. Deflection

Envelope Member Section Stresses

	Member	Sec		Axial[ksi]	LC	y Shear[...	LC	z Shear...	LC	y-Top[ksi]	LC	y-Bot[ksi]	LC	z-To...	LC	z-B...	LC
1	M2	1	max	.002	4	.049	3	0	4	0	1	0	1	0	1	0	1
2			min	0	1	-.037	6	-.003	5	0	1	0	1	0	1	0	1
3		2	max	.002	4	.081	6	.001	5	1.489	6	2.482	3	.004	5	0	4
4			min	0	1	-.139	3	0	4	-2.482	3	-1.489	6	0	4	-.004	5
5		3	max	.002	4	.018	3	0	5	.61	6	.506	3	0	4	0	5
6			min	0	1	.004	6	0	4	-.506	3	-.61	6	0	5	0	4
7		4	max	.002	4	.009	3	0	1	.517	6	.126	3	0	5	0	1
8			min	0	1	-.01	6	0	5	-.126	3	-.517	6	0	1	0	5
9		5	max	.002	4	0	3	0	5	0	1	0	1	0	1	0	1
10			min	0	1	-.025	6	0	1	0	1	0	1	0	1	0	1
11	M3	1	max	0	5	.002	5	0	1	0	1	0	1	0	1	0	1
12			min	0	1	.001	3	0	1	0	1	0	1	0	1	0	1
13		2	max	0	5	.001	5	0	1	.004	5	-.002	3	0	1	0	1
14			min	0	1	0	3	0	1	.002	3	-.004	5	0	1	0	1
15		3	max	0	5	0	1	0	1	.005	5	-.003	3	0	1	0	1

Envelope Member Section Stresses (Continued)

	Member	Sec		Axial[ksi]	LC	y Shear[...	LC	z Shear...	LC	y-Top[ksi]	LC	y-Bot[ksi]	LC	z-To...	LC	z-B...	LC
16			min	0	1	0	1	0	1	.003	3	-.005	5	0	1	0	1
17		4	max	0	5	0	2	0	1	.004	5	-.002	3	0	1	0	1
18			min	0	1	-.001	4	0	1	.002	3	-.004	5	0	1	0	1
19		5	max	0	5	-.001	2	0	1	0	1	0	1	0	1	0	1
20			min	0	1	-.002	4	0	1	0	1	0	1	0	1	0	1
21	M4	1	max	0	1	.002	5	0	1	0	1	0	1	0	1	0	1
22			min	0	5	.001	2	0	1	0	1	0	1	0	1	0	1
23		2	max	0	1	.001	5	0	1	.004	5	-.002	2	0	1	0	1
24			min	0	5	0	2	0	1	.002	2	-.004	5	0	1	0	1
25		3	max	0	1	0	1	0	1	.005	5	-.003	2	0	1	0	1
26			min	0	5	0	1	0	1	.003	2	-.005	5	0	1	0	1
27		4	max	0	1	0	3	0	1	.004	5	-.002	2	0	1	0	1
28			min	0	5	-.001	1	0	1	.002	2	-.004	5	0	1	0	1
29		5	max	0	1	-.001	3	0	1	0	1	0	1	0	1	0	1
30			min	0	5	-.002	1	0	1	0	1	0	1	0	1	0	1
31	M5	1	max	0	5	.034	3	0	4	0	1	0	1	0	1	0	1
32			min	0	4	-.022	6	0	5	0	1	0	1	0	1	0	1
33		2	max	0	5	.033	3	0	4	.083	3	.055	6	0	4	0	5
34			min	0	4	-.023	6	0	5	-.055	6	-.083	3	0	5	0	4
35		3	max	0	5	.033	3	0	4	.164	3	.114	6	0	4	0	5
36			min	0	4	-.024	6	0	5	-.114	6	-.164	3	0	5	0	4
37		4	max	0	5	.032	3	0	4	.244	3	.175	6	0	4	0	5
38			min	0	4	-.025	6	0	5	-.175	6	-.244	3	0	5	0	4
39		5	max	0	5	.031	3	0	4	.322	3	.238	6	0	4	0	5
40			min	0	4	-.026	6	0	5	-.238	6	-.322	3	0	5	0	4
41	M9	1	max	0	1	.041	6	0	1	.05	3	.05	6	0	4	0	1
42			min	0	5	-.06	2	0	4	-.05	6	-.05	3	0	1	0	4
43		2	max	0	1	.023	6	0	4	.387	6	.636	2	0	4	0	1
44			min	0	5	-.036	2	0	1	-.636	2	-.387	6	0	1	0	4
45		3	max	0	1	.015	3	0	1	.503	6	.819	2	.033	4	0	1
46			min	0	5	-.006	6	-.019	4	-.819	2	-.503	6	0	1	-.033	4
47		4	max	0	1	.039	2	.019	4	.354	6	.538	2	0	1	.015	4
48			min	0	5	-.024	6	0	5	-.538	2	-.354	6	-.015	4	0	1
49		5	max	0	1	.049	2	0	5	0	1	0	1	0	1	0	1
50			min	0	5	-.037	6	-.006	4	0	1	0	1	0	1	0	1
51	M16	1	max	0	4	.045	6	0	1	0	1	0	1	0	1	0	1
52			min	0	1	-.106	3	0	1	0	1	0	1	0	1	0	1
53		2	max	0	4	.022	6	0	1	.268	6	.634	2	0	1	0	1
54			min	0	1	-.053	3	0	1	-.634	2	-.268	6	0	1	0	1
55		3	max	0	4	0	1	0	1	.358	6	.845	2	0	1	0	1
56			min	0	1	0	1	0	1	-.845	2	-.358	6	0	1	0	1
57		4	max	0	4	.053	2	0	1	.268	6	.634	2	0	1	0	1
58			min	0	1	-.022	6	0	1	-.634	2	-.268	6	0	1	0	1
59		5	max	0	4	.106	2	0	1	0	1	0	1	0	1	0	1
60			min	0	1	-.045	6	0	1	0	1	0	1	0	1	0	1
61	M24	1	max	0	4	.079	6	0	1	0	1	0	1	0	1	0	1
62			min	0	1	-.186	2	0	1	0	1	0	1	0	1	0	1
63		2	max	0	4	.039	6	0	1	.831	6	1.963	2	0	1	0	1
64			min	0	1	-.093	2	0	1	-1.963	2	-.831	6	0	1	0	1
65		3	max	0	4	0	1	0	1	1.108	6	2.618	2	0	1	0	1
66			min	0	1	0	1	0	1	-2.618	2	-1.108	6	0	1	0	1
67		4	max	0	4	.093	2	0	1	.831	6	1.963	2	0	1	0	1
68			min	0	1	-.039	6	0	1	-1.963	2	-.831	6	0	1	0	1
69		5	max	0	4	.186	2	0	1	0	1	0	1	0	1	0	1
70			min	0	1	-.079	6	0	1	0	1	0	1	0	1	0	1
71	M32	1	max	0	4	.023	6	0	1	0	1	0	1	0	1	0	1
72			min	0	1	-.055	3	0	1	0	1	0	1	0	1	0	1

Envelope Member Section Stresses (Continued)

	Member	Sec		Axial[ksi]	LC	y Shear[...	LC	z Shear...	LC	y-Top[ksi]	LC	y-Bot[ksi]	LC	z-To...	LC	z-B...	LC
73		2	max	0	4	.012	6	0	1	.073	6	.172	3	0	1	0	1
74			min	0	1	-.028	3	0	1	-.172	3	-.073	6	0	1	0	1
75		3	max	0	4	0	1	0	1	.097	6	.23	3	0	1	0	1
76			min	0	1	0	1	0	1	-.23	3	-.097	6	0	1	0	1
77		4	max	0	4	.028	2	0	1	.073	6	.172	3	0	1	0	1
78			min	0	1	-.012	6	0	1	-.172	3	-.073	6	0	1	0	1
79		5	max	0	4	.055	2	0	1	0	1	0	1	0	1	0	1
80			min	0	1	-.023	6	0	1	0	1	0	1	0	1	0	1
81	M31A	1	max	0	1	.018	3	0	1	0	1	0	1	0	1	0	4
82			min	0	1	-.009	6	0	4	0	1	0	1	0	4	0	1
83		2	max	0	1	.032	3	0	1	.104	3	.05	6	0	1	0	4
84			min	0	1	-.015	6	0	4	-.05	6	-.104	3	0	4	0	1
85		3	max	0	1	.045	3	0	1	.265	3	.126	6	0	1	0	4
86			min	0	1	-.021	6	0	4	-.126	6	-.265	3	0	4	0	1
87		4	max	0	1	.059	3	0	1	.482	3	.227	6	0	1	0	4
88			min	0	1	-.028	6	0	4	-.227	6	-.482	3	0	4	0	1
89		5	max	0	1	.072	3	0	1	.756	3	.355	6	0	1	0	4
90			min	0	1	-.034	6	0	4	-.355	6	-.756	3	0	4	0	1
91	M32A	1	max	0	1	.041	6	0	1	.709	2	.32	6	0	4	0	1
92			min	0	1	-.091	2	0	4	-.32	6	-.709	2	0	1	0	4
93		2	max	0	1	.02	6	0	1	.116	6	.246	3	0	4	0	1
94			min	0	1	-.045	2	0	4	-.246	3	-.116	6	0	1	0	4
95		3	max	0	1	0	3	0	1	.256	6	.558	3	0	4	0	1
96			min	0	1	0	6	0	4	-.558	3	-.256	6	0	1	0	4
97		4	max	0	1	.046	3	0	1	.102	6	.227	2	0	1	0	4
98			min	0	1	-.021	6	0	4	-.227	2	-.102	6	0	4	0	1
99		5	max	0	1	.092	3	0	1	.748	3	.348	6	0	1	0	4
100			min	0	1	-.042	6	0	4	-.348	6	-.748	3	0	4	0	1
101	M33	1	max	0	1	.034	6	0	1	.727	2	.329	6	0	4	0	1
102			min	0	1	-.075	2	0	4	-.329	6	-.727	2	0	1	0	4
103		2	max	0	1	.022	6	0	1	.234	2	.104	6	0	4	0	1
104			min	0	1	-.049	2	0	4	-.104	6	-.234	2	0	1	0	4
105		3	max	0	1	.01	6	0	1	.026	6	.052	3	0	4	0	1
106			min	0	1	-.023	2	0	4	-.052	3	-.026	6	0	1	0	4
107		4	max	0	1	.003	3	0	1	.061	6	.13	3	0	4	0	1
108			min	0	1	-.002	6	0	4	-.13	3	-.061	6	0	1	0	4
109		5	max	0	1	.029	3	0	1	0	1	0	1	0	1	0	1
110			min	0	1	-.014	6	0	4	0	1	0	1	0	1	0	1
111	M34	1	max	0	1	.008	6	0	4	.083	6	.111	3	0	1	0	4
112			min	-.041	5	-.045	2	0	1	-.111	3	-.083	6	0	4	0	1
113		2	max	0	1	.035	3	0	1	.006	6	.499	2	0	1	0	4
114			min	-.041	5	-.036	6	0	4	-.499	2	-.006	6	0	4	0	1
115		3	max	0	1	.198	3	0	1	.961	3	1.01	6	.013	4	0	1
116			min	-.041	5	-.114	6	0	4	-1.01	6	-.961	3	0	1	-.013	4
117		4	max	0	5	.117	6	0	4	2.154	3	1.588	6	0	1	.005	4
118			min	0	1	-.163	3	0	1	-1.588	6	-2.154	3	-.005	4	0	1
119		5	max	0	5	.09	6	0	1	0	1	0	1	0	1	0	1
120			min	0	1	-.126	3	0	4	0	1	0	1	0	1	0	1
121	M35	1	max	0	1	.11	6	0	4	.07	3	.042	6	0	1	0	4
122			min	0	5	-.226	3	0	1	-.042	6	-.07	3	0	4	0	1
123		2	max	0	1	.059	6	0	1	1.44	6	3.085	3	0	1	0	4
124			min	0	5	-.128	3	0	4	-3.085	3	-1.44	6	0	4	0	1
125		3	max	0	1	.071	2	0	1	1.797	6	3.964	3	.005	4	0	1
126			min	0	5	-.035	6	0	4	-3.964	3	-1.797	6	0	1	-.005	4
127		4	max	0	1	.168	3	0	4	.896	6	2.197	3	0	1	.002	4
128			min	0	5	-.084	6	0	1	-2.197	3	-.896	6	-.002	4	0	1
129		5	max	0	4	.138	3	0	1	0	1	0	1	0	1	0	1

Envelope Member Section Stresses (Continued)

	Member	Sec		Axial[ksi]	LC	y Shear[...	LC	z Shear...	LC	y-Top[ksi]	LC	y-Bot[ksi]	LC	z-To...	LC	z-B...	LC
130			min	0	5	-.06	6	0	4	0	1	0	1	0	1	0	1
131	M26	1	max	.003	4	.045	6	0	1	0	1	0	1	0	1	0	1
132			min	0	1	-.106	3	0	1	0	1	0	1	0	1	0	1
133		2	max	.003	4	.022	6	0	1	.268	6	.634	2	0	1	0	1
134			min	0	1	-.053	3	0	1	-.634	2	-.268	6	0	1	0	1
135		3	max	.003	4	0	1	0	1	.358	6	.845	2	0	1	0	1
136			min	0	1	0	1	0	1	-.845	2	-.358	6	0	1	0	1
137		4	max	.003	4	.053	2	0	1	.268	6	.634	3	0	1	0	1
138			min	0	1	-.022	6	0	1	-.634	3	-.268	6	0	1	0	1
139		5	max	.003	4	.106	2	0	1	0	1	0	1	0	1	0	1
140			min	0	1	-.045	6	0	1	0	1	0	1	0	1	0	1
141	M27	1	max	.003	4	.079	6	0	1	0	1	0	1	0	1	0	1
142			min	0	1	-.186	2	0	1	0	1	0	1	0	1	0	1
143		2	max	.003	4	.039	6	0	1	.831	6	1.963	2	0	1	0	1
144			min	0	1	-.093	2	0	1	-1.963	2	-.831	6	0	1	0	1
145		3	max	.003	4	0	1	0	1	1.108	6	2.618	2	0	1	0	1
146			min	0	1	0	1	0	1	-2.618	2	-1.108	6	0	1	0	1
147		4	max	.003	4	.093	2	0	1	.831	6	1.963	2	0	1	0	1
148			min	0	1	-.039	6	0	1	-1.963	2	-.831	6	0	1	0	1
149		5	max	.003	4	.186	2	0	1	0	1	0	1	0	1	0	1
150			min	0	1	-.079	6	0	1	0	1	0	1	0	1	0	1
151	M28	1	max	.003	4	.023	6	0	1	0	1	0	1	0	1	0	1
152			min	0	1	-.055	2	0	1	0	1	0	1	0	1	0	1
153		2	max	.003	4	.012	6	0	1	.073	6	.172	2	0	1	0	1
154			min	0	1	-.028	2	0	1	-.172	2	-.073	6	0	1	0	1
155		3	max	.003	4	0	1	0	1	.097	6	.23	2	0	1	0	1
156			min	0	1	0	1	0	1	-.23	2	-.097	6	0	1	0	1
157		4	max	.003	4	.028	2	0	1	.073	6	.172	2	0	1	0	1
158			min	0	1	-.012	6	0	1	-.172	2	-.073	6	0	1	0	1
159		5	max	.003	4	.055	2	0	1	0	1	0	1	0	1	0	1
160			min	0	1	-.023	6	0	1	0	1	0	1	0	1	0	1
161	M32B	1	max	0	1	.045	6	0	1	0	1	0	1	0	1	0	1
162			min	-.015	4	-.106	2	0	1	0	1	0	1	0	1	0	1
163		2	max	0	1	.022	6	0	1	.268	6	.634	2	0	1	0	1
164			min	-.015	4	-.053	2	0	1	-.634	2	-.268	6	0	1	0	1
165		3	max	0	1	0	1	0	1	.358	6	.845	2	0	1	0	1
166			min	-.015	4	0	1	0	1	-.845	2	-.358	6	0	1	0	1
167		4	max	0	1	.053	2	0	1	.268	6	.634	2	0	1	0	1
168			min	-.015	4	-.022	6	0	1	-.634	2	-.268	6	0	1	0	1
169		5	max	0	1	.106	2	0	1	0	1	0	1	0	1	0	1
170			min	-.015	4	-.045	6	0	1	0	1	0	1	0	1	0	1
171	M33A	1	max	0	1	.079	6	0	1	0	1	0	1	0	1	0	1
172			min	-.015	4	-.186	2	0	1	0	1	0	1	0	1	0	1
173		2	max	0	1	.039	6	0	1	.831	6	1.963	2	0	1	0	1
174			min	-.015	4	-.093	2	0	1	-1.963	2	-.831	6	0	1	0	1
175		3	max	0	1	0	1	0	1	1.108	6	2.618	2	0	1	0	1
176			min	-.015	4	0	1	0	1	-2.618	2	-1.108	6	0	1	0	1
177		4	max	0	1	.093	2	0	1	.831	6	1.963	2	0	1	0	1
178			min	-.015	4	-.039	6	0	1	-1.963	2	-.831	6	0	1	0	1
179		5	max	0	1	.186	2	0	1	0	1	0	1	0	1	0	1
180			min	-.015	4	-.079	6	0	1	0	1	0	1	0	1	0	1
181	M34A	1	max	0	1	.023	6	0	1	0	1	0	1	0	1	0	1
182			min	-.014	4	-.055	2	0	1	0	1	0	1	0	1	0	1
183		2	max	0	1	.012	6	0	1	.073	6	.172	2	0	1	0	1
184			min	-.014	4	-.028	2	0	1	-.172	2	-.073	6	0	1	0	1
185		3	max	0	1	0	1	0	1	.097	6	.23	2	0	1	0	1
186			min	-.014	4	0	1	0	1	-.23	2	-.097	6	0	1	0	1

Envelope Member Section Stresses (Continued)

	Member	Sec		Axial[ksi]	LC	y Shear[...	LC	z Shear...	LC	y-Top[ksi]	LC	y-Bot[ksi]	LC	z-To...	LC	z-B...	LC
187		4	max	0	1	.028	2	0	1	.073	6	.172	2	0	1	0	1
188			min	-.014	4	-.012	6	0	1	-.172	2	-.073	6	0	1	0	1
189		5	max	0	1	.055	2	0	1	0	1	0	1	0	1	0	1
190			min	-.014	4	-.023	6	0	1	0	1	0	1	0	1	0	1
191	M35A	1	max	0	1	.045	6	0	1	0	1	0	1	0	1	0	1
192			min	-.124	4	-.106	2	0	1	0	1	0	1	0	1	0	1
193		2	max	0	1	.022	6	0	1	.268	6	.634	2	0	1	0	1
194			min	-.124	4	-.053	2	0	1	-.634	2	-.268	6	0	1	0	1
195		3	max	0	1	0	1	0	1	.358	6	.845	2	0	1	0	1
196			min	-.124	4	0	1	0	1	-.845	2	-.358	6	0	1	0	1
197		4	max	0	1	.053	2	0	1	.268	6	.634	2	0	1	0	1
198			min	-.124	4	-.022	6	0	1	-.634	2	-.268	6	0	1	0	1
199		5	max	0	1	.106	2	0	1	0	1	0	1	0	1	0	1
200			min	-.124	4	-.045	6	0	1	0	1	0	1	0	1	0	1
201	M36	1	max	0	1	.079	6	0	1	0	1	0	1	0	1	0	1
202			min	-.125	4	-.186	2	0	1	0	1	0	1	0	1	0	1
203		2	max	0	1	.039	6	0	1	.831	6	1.963	2	0	1	0	1
204			min	-.125	4	-.093	2	0	1	-1.963	2	-.831	6	0	1	0	1
205		3	max	0	1	0	1	0	1	1.108	6	2.618	2	0	1	0	1
206			min	-.125	4	0	1	0	1	-2.618	2	-1.108	6	0	1	0	1
207		4	max	0	1	.093	2	0	1	.831	6	1.963	2	0	1	0	1
208			min	-.125	4	-.039	6	0	1	-1.963	2	-.831	6	0	1	0	1
209		5	max	0	1	.186	2	0	1	0	1	0	1	0	1	0	1
210			min	-.125	4	-.079	6	0	1	0	1	0	1	0	1	0	1
211	M37	1	max	.021	4	.023	6	0	1	0	1	0	1	0	1	0	1
212			min	0	1	-.055	2	0	1	0	1	0	1	0	1	0	1
213		2	max	.021	4	.012	6	0	1	.073	6	.172	2	0	1	0	1
214			min	0	1	-.028	2	0	1	-.172	2	-.073	6	0	1	0	1
215		3	max	.021	4	0	1	0	1	.097	6			0	1	0	1
216			min	0	1	0	1	0	1	-.23	2			0	1	0	1
217		4	max	.021	4	.028	2	0	1	.073	6			0	1	0	1
218			min	0	1	-.012	6	0	1	-.172	2	-.073	6	0	1	0	1
219		5	max	.021	4	.055	2	0	1	0	1	0	1	0	1	0	1
220			min	0	1	-.023	6	0	1	0	1	0	1	0	1	0	1
221	M38	1	max	0	5	.181	6	0	5	6.915	3	5.033	6	0	4	0	5
222			min	-.015	4	-.268	3	0	4	-5.033	6	-6.915	3	0	5	0	4
223		2	max	0	5	.169	6	0	5	4.874	3	3.632	6	0	4	0	5
224			min	-.015	4	-.242	3	0	4	-3.632	6	-4.874	3	0	5	0	4
225		3	max	0	5	.157	6	0	5	3.042	3	2.326	6	0	4	0	5
226			min	-.015	4	-.216	3	0	4	-2.326	6	-3.042	3	0	5	0	4
227		4	max	0	5	.146	6	0	5	1.417	3	1.115	6	0	4	0	5
228			min	-.015	4	-.19	3	0	4	-1.115	6	-1.417	3	0	5	0	4
229		5	max	0	5	.134	6	0	5	0	1	0	1	0	1	0	1
230			min	-.015	4	-.164	3	0	4	0	1	0	1	0	1	0	1
231	M39	1	max	0	5	.042	6	0	1	0	1	0	1	0	1	0	1
232			min	-.015	4	-.092	2	0	1	0	1	0	1	0	1	0	1
233		2	max	0	5	.021	6	0	1	.443	6	.965	2	0	1	0	1
234			min	-.015	4	-.046	2	0	1	-.965	2	-.443	6	0	1	0	1
235		3	max	0	5	0	1	0	1	.591	6	1.287	2	0	1	0	1
236			min	-.015	4	0	1	0	1	-1.287	2	-.591	6	0	1	0	1
237		4	max	0	5	.046	2	0	1	.443	6	.965	2	0	1	0	1
238			min	-.015	4	-.021	6	0	1	-.965	2	-.443	6	0	1	0	1
239		5	max	0	5	.092	2	0	1	0	1	0	1	0	1	0	1
240			min	-.015	4	-.042	6	0	1	0	1	0	1	0	1	0	1
241	M40	1	max	0	5	.012	6	0	1	0	1	0	1	0	1	0	1
242			min	-.014	4	-.027	2	0	1	0	1	0	1	0	1	0	1
243		2	max	0	5	.006	6	0	1	.039	6	.085	2	0	1	0	1

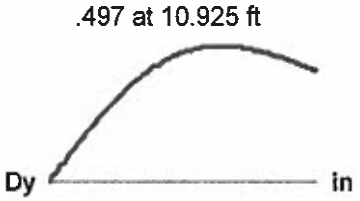
fb < 9000 psi
OK

Envelope Member Section Stresses (Continued)

	Member	Sec		Axial[ksi]	LC	y Shear[...	LC	z Shear...	LC	y-Top[ksi]	LC	y-Bot[ksi]	LC	z-To...	LC	z-B...	LC
244			min	-.014	4	-.014	2	0	1	-.085	2	-.039	6	0	1	0	1
245		3	max	0	5	0	1	0	1	.052	6	.113	2	0	1	0	1
246			min	-.014	4	0	1	0	1	-.113	2	-.052	6	0	1	0	1
247		4	max	0	5	.014	2	0	1	.039	6	.085	2	0	1	0	1
248			min	-.014	4	-.006	6	0	1	-.085	2	-.039	6	0	1	0	1
249		5	max	0	5	.027	2	0	1	0	1	0	1	0	1	0	1
250			min	-.014	4	-.012	6	0	1	0	1	0	1	0	1	0	1
251	M38A	1	max	.42	6	.19	2	0	1	0	1	0	1	0	1	0	1
252			min	-.675	3	0	1	-.19	3	0	1	0	1	0	1	0	1
253		2	max	.427	6	.095	2	0	1	3.001	2	0	1	0	1	3.001	3
254			min	-.671	3	0	1	-.095	3	0	1	-3.001	2	-3.001	3	0	1
255		3	max	.433	6	0	1	0	1	4.001	2	0	1	0	1	4.001	3
256			min	-.668	3	0	1	0	1	0	1	-4.001	2	-4.001	3	0	1
257		4	max	.439	6	0	1	.095	5	3.001	2	0	1	0	1	3.001	3
258			min	-.664	3	-.095	2	0	1	0	1	-3.001	2	-3.001	3	0	1
259		5	max	.446	6	0	1	.19	5	0	1	0	1	0	1	0	1
260			min	-.66	3	-.19	2	0	1	0	1	0	1	0	1	0	1

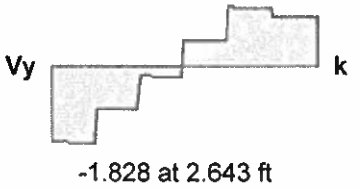
Beam: ***M35***

Shape: **12"X1"**
Material: **Aluminum**
Length: **16.917 ft**
I Joint: **N11**
J Joint: **N10**
LC 3: 0.6DL+Wup+WIND Z
Code Check: **0.708 (bending)**
Report Based On 97 Sections



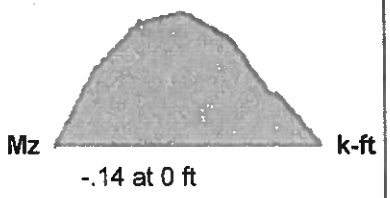
Dz — in

A: -.004 at 11.278 ft / -.004 at 14.097 ft — k



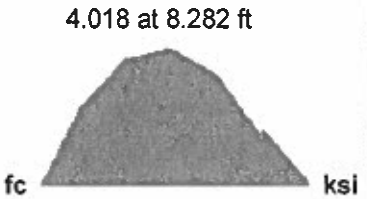
Vz — k

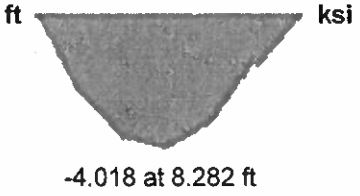
T: -.008 at 0 ft / -.008 at 11.278 ft — k-ft



My — k-ft
0 at 13.921 ft

fa: 0 at 11.278 ft / 0 at 14.097 ft — ksi





AISC 13th ASD Code Check

Direct Analysis Method

Max Bending Check	**0.708**	Max Shear Check	**0.047 (y)**
Location	**8.282 ft**	Location	**2.643 ft**
Equation	**H1-1b**	Max Defl Ratio	**L/753**
Bending Flange	**Compact**	Compression Flange	**Non-Slender**
Bending Web	**Compact**	Compression Web	**Non-Slender**

Fy	**15.03 ksi**
Pnc/om	**45.094 k**
Pnt/om	**108 k**
Mny/om	**2.25 k-ft**
Mnz/om	**11.35 k-ft**
Vny/om	**64.8 k**
Vnz/om	**64.8 k**
Cb	**1.203**

	Y-Y	Z-Z
Lb	**2.83 ft**	**2.83 ft**
KL/r	**117.641**	**9.803**
Sway	**No**	**No**

L Comp Flange	**16.917 ft**
Torque Length	**NC**
Tau_b	**1**

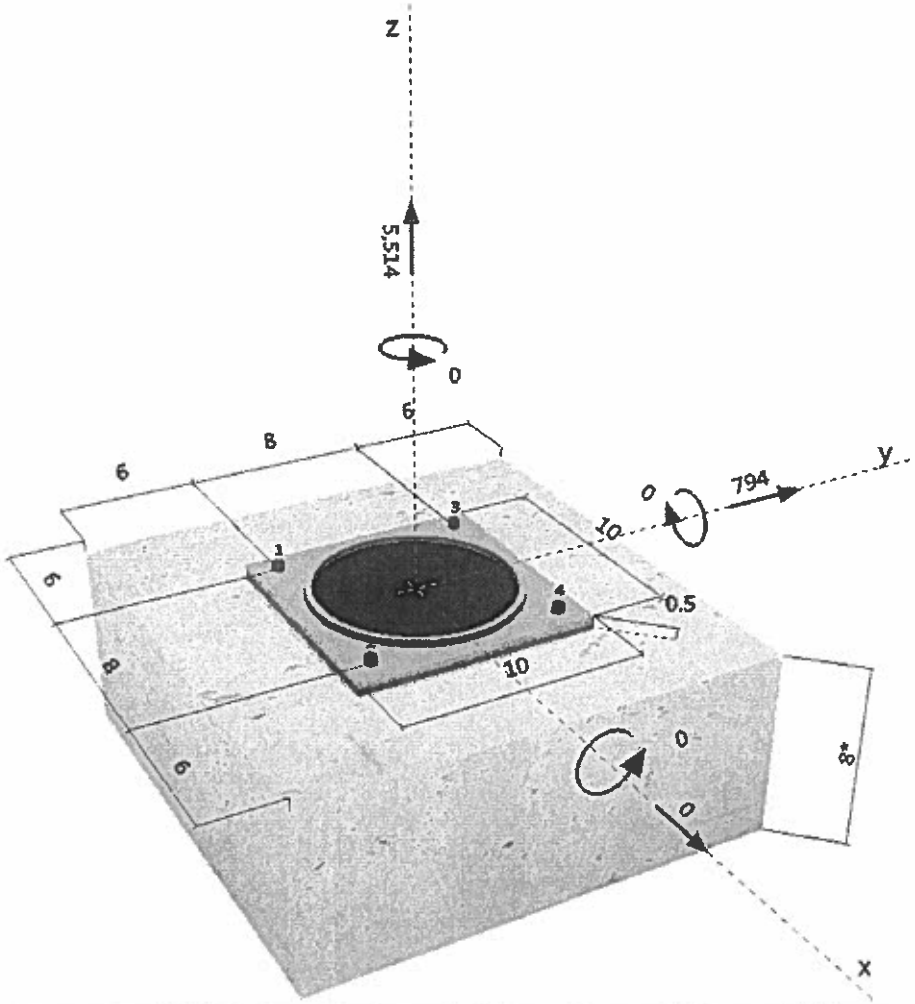
Company:
Specifier:
Address:
Phone I Fax: |
E-Mail:

COLUMN CONNECTION

1 Input data

Anchor type and diameter:	**KWIK HUS-EZ (KH-EZ) 1/2 (3)**
Effective embedment depth:	h_{ef} = 2.160 in., h_{nom} = 3.000 in.
Material:	Carbon Steel
Evaluation Service Report:	ESR-3027
Issued I Valid:	2/1/2016 | 12/1/2017
Proof:	Design method ACI 318 / AC193
Stand-off installation:	e_b = 0.000 in. (no stand-off); t = 0.500 in.
Anchor plate:	l_x x l_y x t = 10.000 in. x 10.000 in. x 0.500 in.; (Recommended plate thickness: not calculated
Profile:	Round HSS, Steel pipe (AISC); (L x W x T) = 8.625 in. x 8.625 in. x 0.250 in.
Base material:	uncracked concrete, 3000, f_c' = 3000 psi; h = 8.000 in.
Reinforcement:	tension: condition B, shear: condition B; no supplemental splitting reinforcement present edge reinforcement: none or < No. 4 bar
Seismic loads (cat. C, D, E, or F)	no

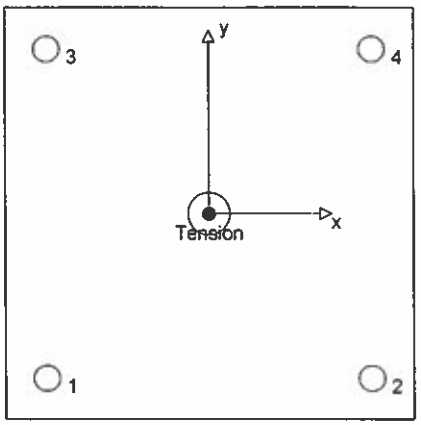
Geometry [in.] & Loading [lb, in.lb]



Company:
Specifier:
Address:
Phone I Fax: |
E-Mail:

2 Load case/Resulting anchor forces

Load case: Design loads



Anchor reactions [lb]
Tension force: (+Tension, -Compression)

Anchor	Tension force	Shear force	Shear force x	Shear force y
1	1378	199	0	199
2	1378	199	0	199
3	1378	199	0	199
4	1378	199	0	199

max. concrete compressive strain: - [‰]
max. concrete compressive stress: - [psi]
resulting tension force in (x/y)=(0.000/0.000): 5514 [lb]
resulting compression force in (x/y)=(0.000/0.000): 0 [lb]

3 Tension load

	Load N_{ua} [lb]	Capacity ϕN_n [lb]	Utilization $\beta_N = N_{ua}/\phi N_n$	Status
Steel Strength*	1378	11778	12	OK
Pullout Strength*	N/A	N/A	N/A	N/A
Concrete Breakout Strength**	5514	12206	46	OK

* anchor having the highest loading **anchor group (anchors in tension)

3.1 Steel Strength

N_{sa} = ESR value refer to ICC-ES ESR-3027
$\phi N_{sa} \geq N_{ua}$ ACI 318-08 Eq. (D-1)

Variables

$A_{se,N}$ [in.^{2}]	f_{uta} [psi]
0.16	112540

Calculations

N_{sa} [lb]
18120

Results

N_{sa} [lb]	ϕ_{steel}	ϕN_{sa} [lb]	N_{ua} [lb]
18120	0.650	11778	1378

Company:
Specifier:
Address:
Phone I Fax:
E-Mail:

3.2 Concrete Breakout Strength

$N_{cbg} = \left(\frac{A_{Nc}}{A_{Nc0}}\right) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b$ ACI 318-08 Eq. (D-5)

$\phi N_{cbg} \geq N_{ua}$ ACI 318-08 Eq. (D-1)

A_{Nc} see ACI 318-08, Part D.5.2.1, Fig. RD.5.2.1(b)

$A_{Nc0} = 9 h_{ef}^2$ ACI 318-08 Eq. (D-6)

$\psi_{ec,N} = \left(\frac{1}{1 + \frac{2 e_N}{3 h_{ef}}}\right) \leq 1.0$ ACI 318-08 Eq. (D-9)

$\psi_{ed,N} = 0.7 + 0.3 \left(\frac{c_{a,min}}{1.5 h_{ef}}\right) \leq 1.0$ ACI 318-08 Eq. (D-11)

$\psi_{cp,N} = MAX\left(\frac{c_{a,min}}{c_{ac}}, \frac{1.5 h_{ef}}{c_{ac}}\right) \leq 1.0$ ACI 318-08 Eq. (D-13)

$N_b = k_c \lambda \sqrt{f'_c} h_{ef}^{1.5}$ ACI 318-08 Eq. (D-7)

Variables

h_{ef} [in.]	$e_{c1,N}$ [in.]	$e_{c2,N}$ [in.]	$c_{a,min}$ [in.]	$\psi_{c,N}$
2.160	0.000	0.000	6.000	1.000

c_{ac} [in.]	k_c	λ	f'_c [psi]
3.750	27	1	3000

Calculations

A_{Nc} [in.^{2}]	A_{Nc0} [in.^{2}]	$\psi_{ec1,N}$	$\psi_{ec2,N}$	$\psi_{ed,N}$	$\psi_{cp,N}$	N_b [lb]
167.96	41.99	1.000	1.000	1.000	1.000	4695

Results

N_{cbg} [lb]	$\phi_{concrete}$	ϕN_{cbg} [lb]	N_{ua} [lb]
18779	0.650	12206	5514

Company:
Specifier:
Address:
Phone I Fax:
E-Mail:

4 Shear load

	Load V_{ua} [lb]	Capacity ϕV_n [lb]	Utilization $\beta_V = V_{ua}/\phi V_n$	Status
Steel Strength*	199	5547	4	OK
Steel failure (with lever arm)*	N/A	N/A	N/A	N/A
Pryout Strength**	794	13145	7	OK
Concrete edge failure in direction y+**	794	5070	16	OK

* anchor having the highest loading **anchor group (relevant anchors)

4.1 Steel Strength

V_{sa} = ESR value refer to ICC-ES ESR-3027
$\phi\ V_{steel} \geq V_{ua}$ ACI 318-08 Eq. (D-2)

Variables

$A_{se,V}$ [in.^{2}]	f_{uta} [psi]
0.16	112540

Calculations

V_{sa} [lb]
9245

Results

V_{sa} [lb]	ϕ_{steel}	$\phi\ V_{sa}$ [lb]	V_{ua} [lb]
9245	0.600	5547	199

4.2 Pryout Strength

$V_{cpg} = k_{cp}\left[\left(\frac{A_{Nc}}{A_{Nc0}}\right)\psi_{ec,N}\psi_{ed,N}\psi_{c,N}\psi_{cp,N}N_b\right]$ ACI 318-08 Eq. (D-31)

$\phi\ V_{cpg} \geq V_{ua}$ ACI 318-08 Eq. (D-2)

A_{Nc} see ACI 318-08, Part D.5.2.1, Fig. RD.5.2.1(b)

$A_{Nc0} = 9\ h_{ef}^2$ ACI 318-08 Eq. (D-6)

$\psi_{ec,N} = \left(\frac{1}{1+\frac{2\ e_N}{3\ h_{ef}}}\right) \leq 1.0$ ACI 318-08 Eq. (D-9)

$\psi_{ed,N} = 0.7 + 0.3\left(\frac{c_{a,min}}{1.5h_{ef}}\right) \leq 1.0$ ACI 318-08 Eq. (D-11)

$\psi_{cp,N} = MAX\left(\frac{c_{a,min}}{c_{ac}}, \frac{1.5h_{ef}}{c_{ac}}\right) \leq 1.0$ ACI 318-08 Eq. (D-13)

$N_b = k_c\ \lambda\ \sqrt{f'_c}\ h_{ef}^{1.5}$ ACI 318-08 Eq. (D-7)

Variables

k_{cp}	h_{ef} [in.]	$e_{c1,N}$ [in.]	$e_{c2,N}$ [in.]	$c_{a,min}$ [in.]
1	2.160	0.000	0.000	6.000

$\psi_{c,N}$	c_{ac} [in.]	k_c	λ	f'_c [psi]
1.000	3.750	27	1	3000

Calculations

A_{Nc} [in.^{2}]	A_{Nc0} [in.^{2}]	$\psi_{ec1,N}$	$\psi_{ec2,N}$	$\psi_{ed,N}$	$\psi_{cp,N}$	N_b [lb]
167.96	41.99	1.000	1.000	1.000	1.000	4695

Results

V_{cpg} [lb]	$\phi_{concrete}$	$\phi\ V_{cpg}$ [lb]	V_{ua} [lb]
18779	0.700	13145	794

Company:
Specifier:
Address:
Phone I Fax:
E-Mail:

4.3 Concrete edge failure in direction y+

$V_{cbg} = \left(\frac{A_{Vc}}{A_{Vc0}}\right) \Psi_{ec,V} \Psi_{ed,V} \Psi_{c,V} \Psi_{h,V} \Psi_{parallel,V} V_b$ ACI 318-08 Eq. (D-22)

$\phi V_{cbg} \geq V_{ua}$ ACI 318-08 Eq. (D-2)

A_{Vc} see ACI 318-08, Part D.6.2.1, Fig. RD.6.2.1(b)

$A_{Vc0} = 4.5 c_{a1}^2$ ACI 318-08 Eq. (D-23)

$\Psi_{ec,V} = \left(\frac{1}{1 + \frac{2e_v}{3c_{a1}}}\right) \leq 1.0$ ACI 318-08 Eq. (D-26)

$\Psi_{ed,V} = 0.7 + 0.3\left(\frac{c_{a2}}{1.5c_{a1}}\right) \leq 1.0$ ACI 318-08 Eq. (D-28)

$\Psi_{h,V} = \sqrt{\frac{1.5c_{a1}}{h_a}} \geq 1.0$ ACI 318-08 Eq. (D-29)

$V_b = \left(7\left(\frac{l_e}{d_a}\right)^{0.2}\sqrt{d_a}\right) \lambda \sqrt{f'_c}\, c_{a1}^{1.5}$ ACI 318-08 Eq. (D-24)

Variables

c_{a1} [in.]	c_{a2} [in.]	e_{cV} [in.]	$\Psi_{c,V}$	h_a [in.]
5.333	6.000	0.000	1.400	8.000

l_e [in.]	λ	d_a [in.]	f'_c [psi]	$\Psi_{parallel,V}$
2.160	1.000	0.500	3000	1.000

Calculations

A_{Vc} [in.^{2}]	A_{Vc0} [in.^{2}]	$\Psi_{ec,V}$	$\Psi_{ed,V}$	$\Psi_{h,V}$	V_b [lb]
160.00	128.00	1.000	0.925	1.000	4474

Results

V_{cbg} [lb]	$\phi_{concrete}$	ϕV_{cbg} [lb]	V_{ua} [lb]
7243	0.700	5070	794

5 Combined tension and shear loads

β_N	β_V	ζ	Utilization $\beta_{N,V}$ [%]	Status
0.452	0.157	5/3	32	OK

$\beta_{NV} = \beta_N^{\zeta} + \beta_V^{\zeta} <= 1$

6 Warnings

- Load re-distributions on the anchors due to elastic deformations of the anchor plate are not considered. The anchor plate is assumed to be sufficiently stiff, in order not to be deformed when subjected to the loading! Input data and results must be checked for agreement with the existing conditions and for plausibility!
- Condition A applies when supplementary reinforcement is used. The Φ factor is increased for non-steel Design Strengths except Pullout Strength and Pryout strength. Condition B applies when supplementary reinforcement is not used and for Pullout Strength and Pryout Strength. Refer to your local standard.
- Refer to the manufacturer's product literature for cleaning and installation instructions.
- Checking the transfer of loads into the base material and the shear resistance are required in accordance with ACI 318 or the relevant standard!

Fastening meets the design criteria!

Company:
Specifier:
Address:
Phone I Fax:
E-Mail:

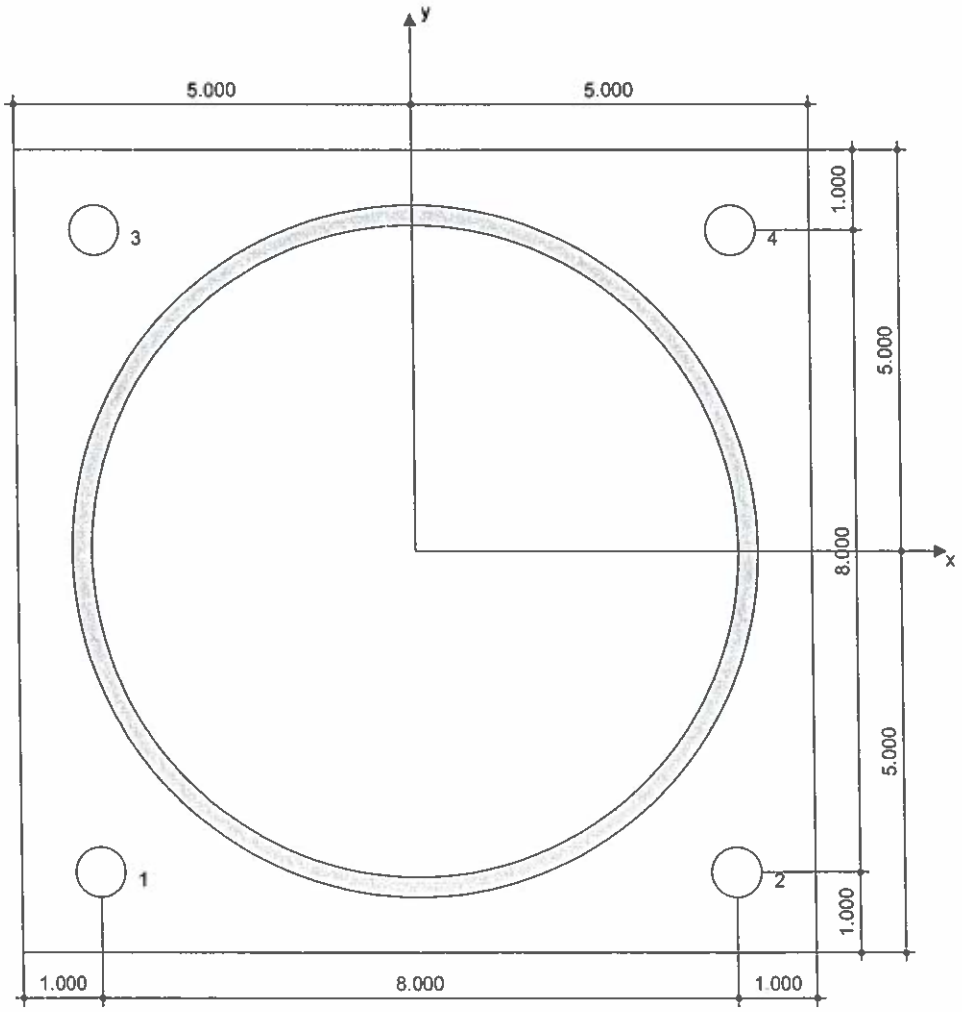
7 Installation data

Anchor plate, steel: -
Profile: Round HSS, Steel pipe (AISC); 8.625 x 8.625 x 0.250 in.
Hole diameter in the fixture: d_f = 0.625 in.
Plate thickness (input): 0.500 in.
Recommended plate thickness: not calculated
Drilling method: Hammer drilled
Cleaning: Manual cleaning of the drilled hole according to instructions for use is required.

Anchor type and diameter: KWIK HUS-EZ (KH-EZ) 1/2 (3)
Installation torque: 540.001 in.lb
Hole diameter in the base material: 0.500 in.
Hole depth in the base material: 3.375 in.
Minimum thickness of the base material: 4.750 in.

7.1 Recommended accessories

Drilling	Cleaning	Setting
• Suitable Rotary Hammer	• Manual blow-out pump	• Torque wrench
• Properly sized drill bit		



Coordinates Anchor in.

Anchor	x	y	c_{-x}	c_{+x}	c_{-y}	c_{+y}
1	-4.000	-4.000	6.000	14.000	6.000	14.000
2	4.000	-4.000	14.000	6.000	6.000	14.000
3	-4.000	4.000	6.000	14.000	14.000	6.000
4	4.000	4.000	14.000	6.000	14.000	6.000

Company:		Page:	7
Specifier:		Project:	
Address:		Sub-Project I Pos. No.:	
Phone I Fax:	|	Date:	5/1/2019
E-Mail:			

8 Remarks; Your Cooperation Duties

- Any and all information and data contained in the Software concern solely the use of Hilti products and are based on the principles, formulas and security regulations in accordance with Hilti's technical directions and operating, mounting and assembly instructions, etc., that must be strictly complied with by the user. All figures contained therein are average figures, and therefore use-specific tests are to be conducted prior to using the relevant Hilti product. The results of the calculations carried out by means of the Software are based essentially on the data you put in. Therefore, you bear the sole responsibility for the absence of errors, the completeness and the relevance of the data to be put in by you. Moreover, you bear sole responsibility for having the results of the calculation checked and cleared by an expert, particularly with regard to compliance with applicable norms and permits, prior to using them for your specific facility. The Software serves only as an aid to interpret norms and permits without any guarantee as to the absence of errors, the correctness and the relevance of the results or suitability for a specific application.
- You must take all necessary and reasonable steps to prevent or limit damage caused by the Software. In particular, you must arrange for the regular backup of programs and data and, if applicable, carry out the updates of the Software offered by Hilti on a regular basis. If you do not use the AutoUpdate function of the Software, you must ensure that you are using the current and thus up-to-date version of the Software in each case by carrying out manual updates via the Hilti Website. Hilti will not be liable for consequences, such as the recovery of lost or damaged data or programs, arising from a culpable breach of duty by you.

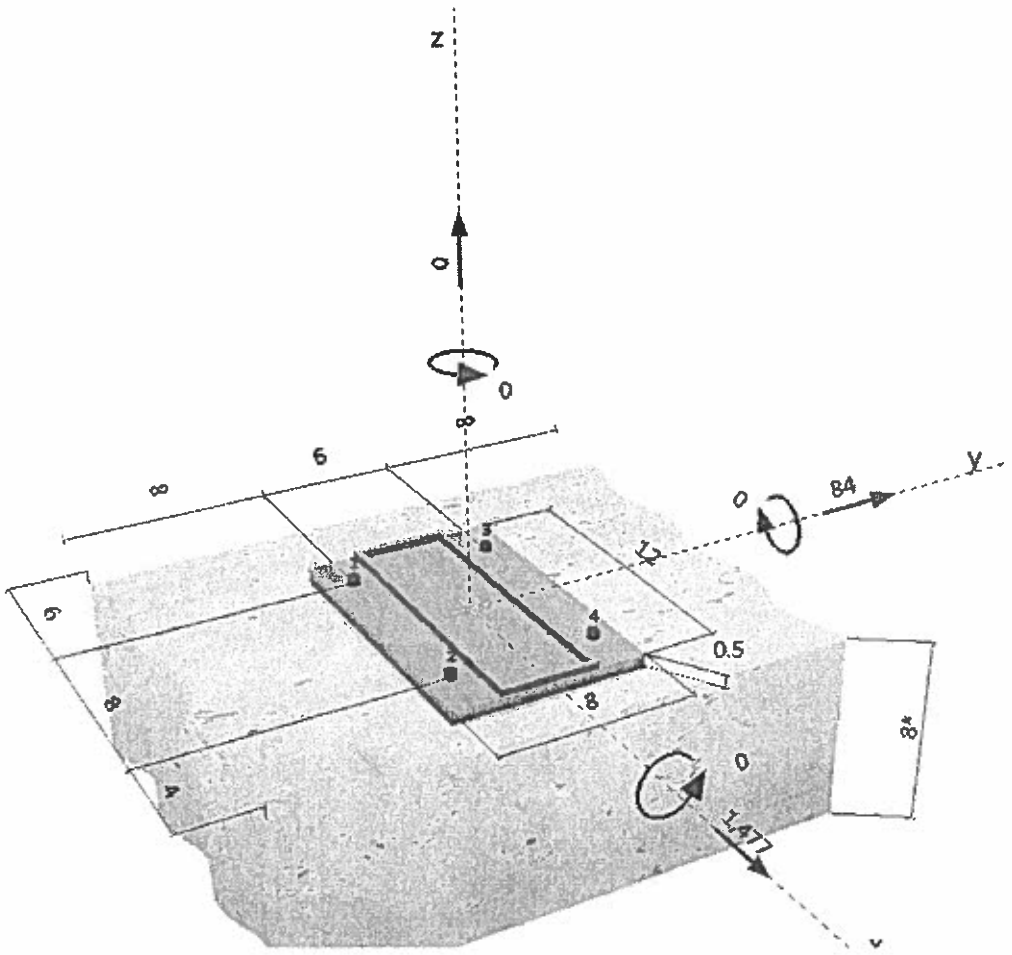
Company:
Specifier:
Address:
Phone I Fax: |
E-Mail:

BEAM CONNECTION

1 Input data

Anchor type and diameter:	KWIK HUS-EZ (KH-EZ) 1/2 (3)
Effective embedment depth:	h_{ef} = 2.160 in., h_{nom} = 3.000 in.
Material:	Carbon Steel
Evaluation Service Report:	ESR-3027
Issued I Valid:	2/1/2016 | 12/1/2017
Proof:	Design method ACI 318 / AC193
Stand-off installation:	e_b = 0.000 in. (no stand-off); t = 0.500 in.
Anchor plate:	l_x x l_y x t = 12.000 in. x 8.000 in. x 0.500 in.; (Recommended plate thickness: not calculated
Profile:	Rectangular HSS (AISC); (L x W x T) = 12.000 in. x 4.000 in. x 0.250 in.
Base material:	uncracked concrete, 3000, f_c' = 3000 psi; h = 8.000 in.
Reinforcement:	tension: condition B, shear: condition B; no supplemental splitting reinforcement present edge reinforcement: none or < No. 4 bar
Seismic loads (cat. C, D, E, or F)	no

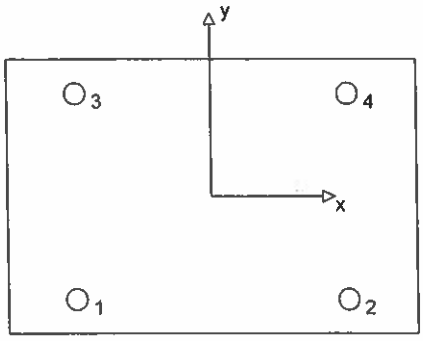
Geometry [in.] & Loading [lb, in.lb]



Company:
Specifier:
Address:
Phone I Fax:
E-Mail:

2 Load case/Resulting anchor forces

Load case: Design loads



Anchor reactions [lb]
Tension force: (+Tension, -Compression)

Anchor	Tension force	Shear force	Shear force x	Shear force y
1	0	370	369	21
2	0	370	369	21
3	0	370	369	21
4	0	370	369	21

max. concrete compressive strain: - [‰]
max. concrete compressive stress: - [psi]
resulting tension force in (x/y)=(0.000/0.000): 0 [lb]
resulting compression force in (x/y)=(0.000/0.000): 0 [lb]

3 Tension load

	Load N_{ua} [lb]	Capacity ϕN_n [lb]	Utilization $\beta_N = N_{ua}/\phi N_n$	Status
Steel Strength*	N/A	N/A	N/A	N/A
Pullout Strength*	N/A	N/A	N/A	N/A
Concrete Breakout Strength**	N/A	N/A	N/A	N/A

* anchor having the highest loading **anchor group (anchors in tension)

Company:
Specifier:
Address:
Phone I Fax:
E-Mail:

4 Shear load

	Load V_{ua} [lb]	Capacity ϕV_n [lb]	Utilization $\beta_V = V_{ua}/\phi V_n$	Status
Steel Strength*	370	5547	7	OK
Steel failure (with lever arm)*	N/A	N/A	N/A	N/A
Pryout Strength**	1479	12658	12	OK
Concrete edge failure in direction x+**	1479	4272	35	OK

* anchor having the highest loading **anchor group (relevant anchors)

4.1 Steel Strength

V_{sa} = ESR value refer to ICC-ES ESR-3027
$\phi V_{steel} \geq V_{ua}$ ACI 318-08 Eq. (D-2)

Variables

$A_{se,V}$ [in.^{2}]	f_{uta} [psi]
0.16	112540

Calculations

V_{sa} [lb]
9245

Results

V_{sa} [lb]	ϕ_{steel}	ϕV_{sa} [lb]	V_{ua} [lb]
9245	0.600	5547	370

4.2 Pryout Strength

$V_{cpg} = k_{cp}\left[\left(\frac{A_{Nc}}{A_{Nc0}}\right)\Psi_{ec,N}\Psi_{ed,N}\Psi_{c,N}\Psi_{cp,N}N_b\right]$ ACI 318-08 Eq. (D-31)

$\phi V_{cpg} \geq V_{ua}$ ACI 318-08 Eq. (D-2)

A_{Nc} see ACI 318-08, Part D.5.2.1, Fig. RD.5.2.1(b)

$A_{Nc0} = 9 h_{ef}^2$ ACI 318-08 Eq. (D-6)

$\Psi_{ec,N} = \left(\frac{1}{1+\frac{2e_N}{3h_{ef}}}\right) \leq 1.0$ ACI 318-08 Eq. (D-9)

$\Psi_{ed,N} = 0.7 + 0.3\left(\frac{c_{a,min}}{1.5h_{ef}}\right) \leq 1.0$ ACI 318-08 Eq. (D-11)

$\Psi_{cp,N} = MAX\left(\frac{c_{a,min}}{c_{ac}}, \frac{1.5h_{ef}}{c_{ac}}\right) \leq 1.0$ ACI 318-08 Eq. (D-13)

$N_b = k_c \lambda \sqrt{f'_c} h_{ef}^{1.5}$ ACI 318-08 Eq. (D-7)

Variables

k_{cp}	h_{ef} [in.]	$e_{c1,N}$ [in.]	$e_{c2,N}$ [in.]	$c_{a,min}$ [in.]
1	2.160	0.000	0.000	4.000

$\Psi_{c,N}$	c_{ac} [in.]	k_c	λ	f'_c [psi]
1.000	3.750	27	1	3000

Calculations

A_{Nc} [in.^{2}]	A_{Nc0} [in.^{2}]	$\Psi_{ec1,N}$	$\Psi_{ec2,N}$	$\Psi_{ed,N}$	$\Psi_{cp,N}$	N_b [lb]
161.74	41.99	1.000	1.000	1.000	1.000	4695

Results

V_{cpg} [lb]	$\phi_{concrete}$	ϕV_{cpg} [lb]	V_{ua} [lb]
18083	0.700	12658	1479

Company:
Specifier:
Address:
Phone I Fax: |
E-Mail:

4.3 Concrete edge failure in direction x+

$V_{cbg} = \left(\frac{A_{Vc}}{A_{Vc0}}\right) \Psi_{ec,V} \Psi_{ed,V} \Psi_{c,V} \Psi_{h,V} \Psi_{parallel,V} V_b$ ACI 318-08 Eq. (D-22)

$\phi V_{cbg} \geq V_{ua}$ ACI 318-08 Eq. (D-2)

A_{Vc} see ACI 318-08, Part D.6.2.1, Fig. RD.6.2.1(b)

$A_{Vc0} = 4.5 c_{a1}^2$ ACI 318-08 Eq. (D-23)

$\Psi_{ec,V} = \left(\frac{1}{1 + \frac{2e_v}{3c_{a1}}}\right) \leq 1.0$ ACI 318-08 Eq. (D-26)

$\Psi_{ed,V} = 0.7 + 0.3\left(\frac{c_{a2}}{1.5c_{a1}}\right) \leq 1.0$ ACI 318-08 Eq. (D-28)

$\Psi_{h,V} = \sqrt{\frac{1.5c_{a1}}{h_a}} \geq 1.0$ ACI 318-08 Eq. (D-29)

$V_b = \left(7\left(\frac{l_e}{d_a}\right)^{0.2} \sqrt{d_a}\right) \lambda \sqrt{f_c'} c_{a1}^{1.5}$ ACI 318-08 Eq. (D-24)

Variables

c_{a1} [in.]	c_{a2} [in.]	e_{cV} [in.]	$\Psi_{c,V}$	h_a [in.]
4.000	-	0.000	1.400	8.000

l_e [in.]	λ	d_a [in.]	f_c' [psi]	$\Psi_{parallel,V}$
2.160	1.000	0.500	3000	1.000

Calculations

A_{Vc} [in.^{2}]	A_{Vc0} [in.^{2}]	$\Psi_{ec,V}$	$\Psi_{ed,V}$	$\Psi_{h,V}$	V_b [lb]
108.00	72.00	1.000	1.000	1.000	2906

Results

V_{cbg} [lb]	$\phi_{concrete}$	ϕV_{cbg} [lb]	V_{ua} [lb]
6103	0.700	4272	1479

5 Warnings

- Load re-distributions on the anchors due to elastic deformations of the anchor plate are not considered. The anchor plate is assumed to be sufficiently stiff, in order not to be deformed when subjected to the loading! Input data and results must be checked for agreement with the existing conditions and for plausibility!
- Condition A applies when supplementary reinforcement is used. The Φ factor is increased for non-steel Design Strengths except Pullout Strength and Pryout strength. Condition B applies when supplementary reinforcement is not used and for Pullout Strength and Pryout Strength. Refer to your local standard.
- Refer to the manufacturer's product literature for cleaning and installation instructions.
- Checking the transfer of loads into the base material and the shear resistance are required in accordance with ACI 318 or the relevant standard!

Fastening meets the design criteria!

Company:
Specifier:
Address:
Phone I Fax:
E-Mail:

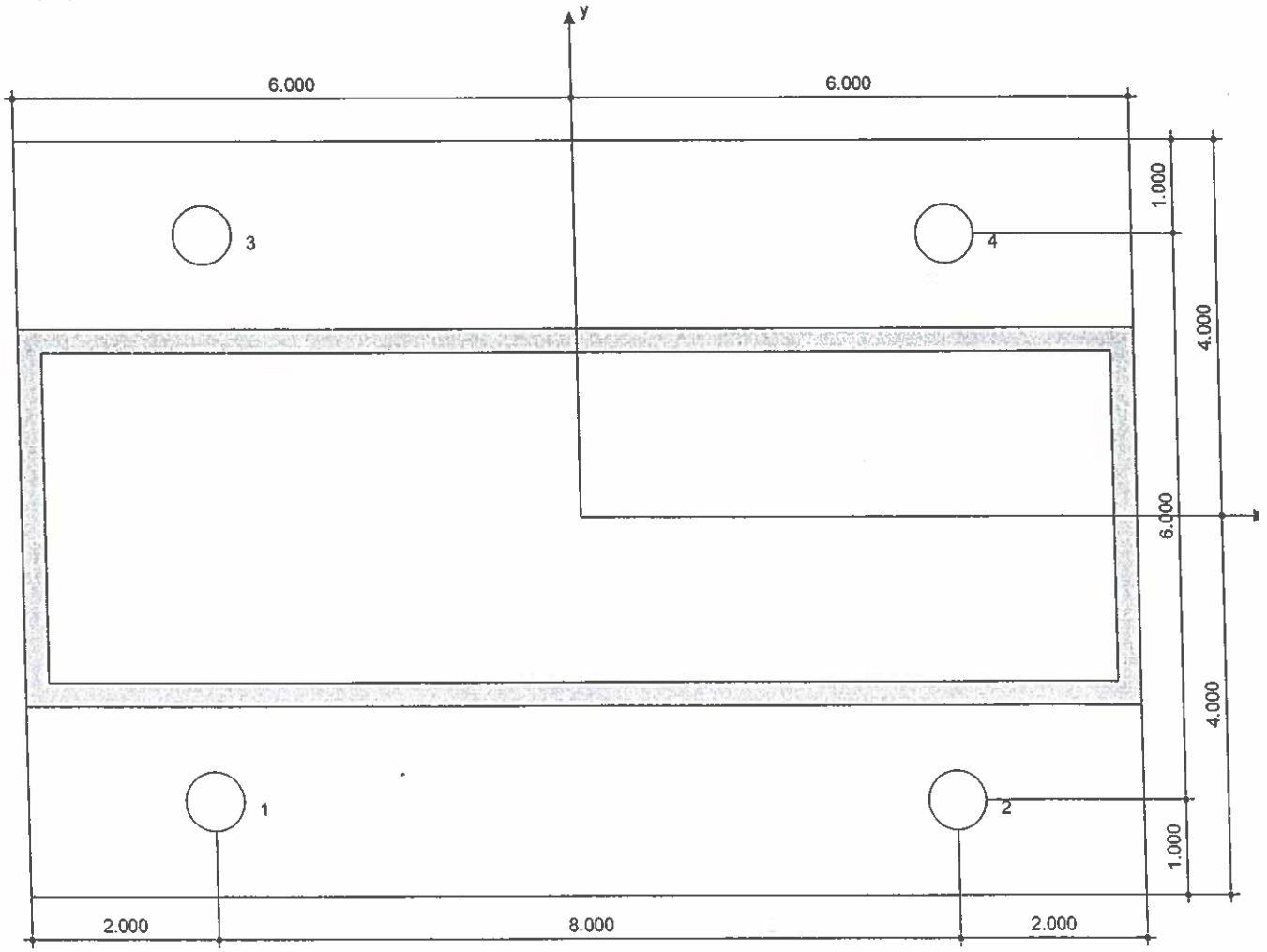
6 Installation data

Anchor plate, steel: -
Profile: Rectangular HSS (AISC); 12.000 x 4.000 x 0.250 in.
Hole diameter in the fixture: d_f = 0.625 in.
Plate thickness (input): 0.500 in.
Recommended plate thickness: not calculated
Drilling method: Hammer drilled
Cleaning: Manual cleaning of the drilled hole according to instructions for use is required.

Anchor type and diameter: KWIK HUS-EZ (KH-EZ) 1/2 (3)
Installation torque: 540.001 in.lb
Hole diameter in the base material: 0.500 in.
Hole depth in the base material: 3.375 in.
Minimum thickness of the base material: 4.750 in.

6.1 Recommended accessories

Drilling	Cleaning	Setting
• Suitable Rotary Hammer	• Manual blow-out pump	• Torque wrench
• Properly sized drill bit		



Coordinates Anchor in.

Anchor	x	y	c_{-x}	c_{+x}	c_{-y}	c_{+y}
1	-4.000	-3.000	6.000	12.000	-	-

7 Remarks; Your Cooperation Duties

- Any and all information and data contained in the Software concern solely the use of Hilti products and are based on the principles, formulas and security regulations in accordance with Hilti's technical directions and operating, mounting and assembly instructions, etc., that must be strictly complied with by the user. All figures contained therein are average figures, and therefore use-specific tests are to be conducted prior to using the relevant Hilti product. The results of the calculations carried out by means of the Software are based essentially on the data you put in. Therefore, you bear the sole responsibility for the absence of errors, the completeness and the relevance of the data to be put in by you. Moreover, you bear sole responsibility for having the results of the calculation checked and cleared by an expert, particularly with regard to compliance with applicable norms and permits, prior to using them for your specific facility. The Software serves only as an aid to interpret norms and permits without any guarantee as to the absence of errors, the correctness and the relevance of the results or suitability for a specific application.
- You must take all necessary and reasonable steps to prevent or limit damage caused by the Software. In particular, you must arrange for the regular backup of programs and data and, if applicable, carry out the updates of the Software offered by Hilti on a regular basis. If you do not use the AutoUpdate function of the Software, you must ensure that you are using the current and thus up-to-date version of the Software in each case by carrying out manual updates via the Hilti Website. Hilti will not be liable for consequences, such as the recovery of lost or damaged data or programs, arising from a culpable breach of duty by you.

Project Name:19-0207 _ 399 Golden Beach Trellis **Date:5/1/2019**

Weld Design For Tube

MATERIALS:

Welding: Aluminum Filler Alloy 5356

Allowable Stress for Welding:
Fv= 7.00 Ksi

Weld and Section Data:

$F_v := 7.00$ Allowable Stress for Welding (ksi)

$b := 4.00$ Width of Tube Steel (in)

$d := 12.00$ Depth of Tube Steel (in)

Load Data:

$M_{weld} := 0.0$ Moment (Kips-ft)

$V_{weld} := 0.925$ Shear (Kips)

$P_{weld} := 0.00$ Tension or Compression (Kips)

$T_{weld} := 0.00$ Torsional Moment (kips-ft)

Check Weld Section :

$$t_e := \left| x \leftarrow 0.00001 \right.$$
$$\text{while } \sqrt{\left[\frac{P_{weld}}{2(b+d)\cdot x} + \frac{M_{weld}\cdot 12}{\left(b\cdot d + \frac{d^2}{3}\right)\cdot x}\right]^2 + \left[\frac{1.5V_{weld}}{2(b+d)\cdot x} + \frac{T_{weld}\cdot 12\cdot\sqrt{\left(\frac{b}{2}\right)^2+\left(\frac{d}{2}\right)^2}}{\frac{(b+d)^3}{6}\cdot x}\right]^2} \geq F_v$$
$$\left. x \leftarrow x + 0.01 \right.$$

$t_e = 0.01$ in

$$S_{weld} := \left(b\cdot d + \frac{d^2}{3}\right)\cdot t_e$$ $S_{weld} = 0.96$ in^3

$$A_{weld} := 2\cdot(d+b)\cdot t_e$$ $A_{weld} = 0.32$ in^2

$$I_{...} := \frac{(b+d)^3}{...}\cdot t$$ $I_{...} = 6.83$ in^3

Check Bending Stress :

$$f_b := \frac{M_{weld} \cdot (12)}{S_{weld}}$$

$f_b = 0.00$ ksi

Check Shear Stress :

$$f_v := \frac{1.5 V_{weld}}{A_{weld}} + \frac{T_{weld} \cdot 12 \cdot \sqrt{\left(\frac{b}{2}\right)^2 + \left(\frac{d}{2}\right)^2}}{J_{weld}}$$

$f_v = 4.33$ ksi

Check Tension Stress :

$$f_a := \frac{P_{weld}}{A_{weld}}$$

$f_a = 0.00$ Ksi

Check Combined Stress :

$$f_{weld} := \sqrt{(f_b + f_a)^2 + f_v^2}$$

$f_{weld} = 4.33$ ksi

$$STRESS := \begin{cases} \text{"N.G."} \\ \text{"OK"} \quad \text{if } F_v \geq f_{weld} \end{cases}$$

STRESS = "OK"

$$W_{weld} := \begin{cases} \text{"3/16"} \quad \text{if } \frac{t_e}{0.707} \leq 0.1875 \\ \text{" 1/4"} \quad \text{if } 0.1875 < \frac{t_e}{0.707} \leq 0.25 \\ \text{"3/8"} \quad \text{if } 0.25 < \frac{t_e}{0.707} \leq 0.375 \\ \text{"1/2"} \quad \text{if } 0.375 < \frac{t_e}{0.707} \leq 0.50 \\ \text{"5/8"} \quad \text{if } 0.50 < \frac{t_e}{0.707} \leq 0.625 \\ \text{"3/4"} \quad \text{if } 0.625 < \frac{t_e}{0.707} \leq 0.750 \\ \text{"1"} \quad \text{if } 0.75 < \frac{t_e}{0.707} \leq 1.00 \end{cases}$$

$W_{weld} = \text{"3/16"}$

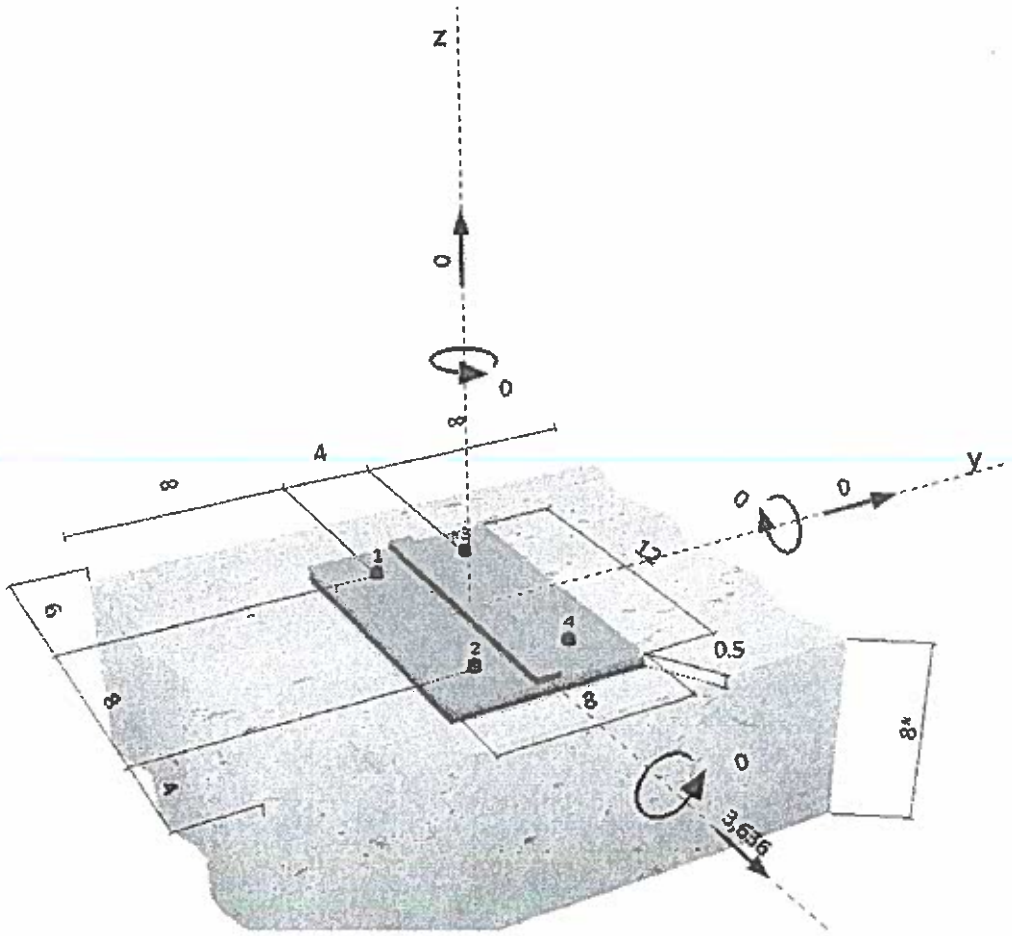
Company:
Specifier:
Address:
Phone I Fax: |
E-Mail:

PLATE CONNECTION

1 Input data

Anchor type and diameter:	KWIK HUS-EZ (KH-EZ) 1/2 (3)
Effective embedment depth:	h_{ef} = 2.160 in., h_{nom} = 3.000 in.
Material:	Carbon Steel
Evaluation Service Report:	ESR-3027
Issued I Valid:	2/1/2016 | 12/1/2017
Proof:	Design method ACI 318 / AC193
Stand-off installation:	e_b = 0.000 in. (no stand-off); t = 0.500 in.
Anchor plate:	l_x x l_y x t = 12.000 in. x 8.000 in. x 0.500 in.; (Recommended plate thickness: not calculated
Profile:	Rectangular plates and bars (AISC); (L x W x T) = 12.000 in. x 1.000 in. x 0.000 in.
Base material:	uncracked concrete, 3000, f_c' = 3000 psi; h = 8.000 in.
Reinforcement:	tension: condition B, shear: condition B; no supplemental splitting reinforcement present edge reinforcement: none or < No. 4 bar
Seismic loads (cat. C, D, E, or F)	no

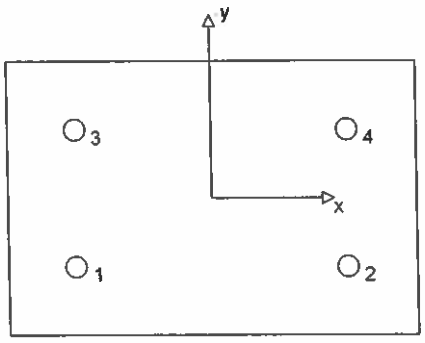
Geometry [in.] & Loading [lb, in.lb]



Company:
Specifier:
Address:
Phone I Fax:
E-Mail:

2 Load case/Resulting anchor forces

Load case: Design loads



Anchor reactions [lb]
Tension force: (+Tension, -Compression)

Anchor	Tension force	Shear force	Shear force x	Shear force y
1	0	909	909	0
2	0	909	909	0
3	0	909	909	0
4	0	909	909	0

max. concrete compressive strain: - [‰]
max. concrete compressive stress: - [psi]
resulting tension force in (x/y)=(0.000/0.000): 0 [lb]
resulting compression force in (x/y)=(0.000/0.000): 0 [lb]

3 Tension load

	Load N_{ua} [lb]	Capacity ϕN_n [lb]	Utilization $\beta_N = N_{ua}/\phi N_n$	Status
Steel Strength*	N/A	N/A	N/A	N/A
Pullout Strength*	N/A	N/A	N/A	N/A
Concrete Breakout Strength**	N/A	N/A	N/A	N/A

* anchor having the highest loading **anchor group (anchors in tension)

Company:
Specifier:
Address:
Phone I Fax:
E-Mail:

4 Shear load

	Load V_{ua} [lb]	Capacity ϕV_n [lb]	Utilization $\beta_V = V_{ua}/\phi V_n$	Status
Steel Strength*	909	5547	17	OK
Steel failure (with lever arm)*	N/A	N/A	N/A	N/A
Pryout Strength**	3636	10630	35	OK
Concrete edge failure in direction x+**	3636	3797	96	OK

* anchor having the highest loading **anchor group (relevant anchors)

4.1 Steel Strength

V_{sa} = ESR value refer to ICC-ES ESR-3027
$\phi V_{steel} \geq V_{ua}$ ACI 318-08 Eq. (D-2)

Variables

$A_{se,V}$ [in.^{2}]	f_{uta} [psi]
0.16	112540

Calculations

V_{sa} [lb]
9245

Results

V_{sa} [lb]	ϕ_{steel}	ϕV_{sa} [lb]	V_{ua} [lb]
9245	0.600	5547	909

4.2 Pryout Strength

$V_{cpg} = k_{cp}\left[\left(\frac{A_{Nc}}{A_{Nc0}}\right)\psi_{ec,N}\psi_{ed,N}\psi_{c,N}\psi_{cp,N}N_b\right]$ ACI 318-08 Eq. (D-31)

$\phi V_{cpg} \geq V_{ua}$ ACI 318-08 Eq. (D-2)

A_{Nc} see ACI 318-08, Part D.5.2.1, Fig. RD.5.2.1(b)

$A_{Nc0} = 9 h_{ef}^2$ ACI 318-08 Eq. (D-6)

$\psi_{ec,N} = \left(\frac{1}{1+\frac{2e_N}{3h_{ef}}}\right) \leq 1.0$ ACI 318-08 Eq. (D-9)

$\psi_{ed,N} = 0.7 + 0.3\left(\frac{c_{a,min}}{1.5h_{ef}}\right) \leq 1.0$ ACI 318-08 Eq. (D-11)

$\psi_{cp,N} = MAX\left(\frac{c_{a,min}}{c_{ac}}, \frac{1.5h_{ef}}{c_{ac}}\right) \leq 1.0$ ACI 318-08 Eq. (D-13)

$N_b = k_c \lambda \sqrt{f'_c}\, h_{ef}^{1.5}$ ACI 318-08 Eq. (D-7)

Variables

k_{cp}	h_{ef} [in.]	$e_{c1,N}$ [in.]	$e_{c2,N}$ [in.]	$c_{a,min}$ [in.]
1	2.160	0.000	0.000	4.000

$\psi_{c,N}$	c_{ac} [in.]	k_c	λ	f'_c [psi]
1.000	3.750	27	1	3000

Calculations

A_{Nc} [in.^{2}]	A_{Nc0} [in.^{2}]	$\psi_{ec1,N}$	$\psi_{ec2,N}$	$\psi_{ed,N}$	$\psi_{cp,N}$	N_b [lb]
135.82	41.99	1.000	1.000	1.000	1.000	4695

Results

V_{cpg} [lb]	$\phi_{concrete}$	ϕV_{cpg} [lb]	V_{ua} [lb]
15185	0.700	10630	3636

Company:
Specifier:
Address:
Phone I Fax:
E-Mail:

4.3 Concrete edge failure in direction x+

$V_{cbg} = \left(\frac{A_{Vc}}{A_{Vc0}}\right) \Psi_{ec,V} \Psi_{ed,V} \Psi_{c,V} \Psi_{h,V} \Psi_{parallel,V} V_b$ ACI 318-08 Eq. (D-22)

$\phi V_{cbg} \geq V_{ua}$ ACI 318-08 Eq. (D-2)

A_{Vc} see ACI 318-08, Part D 6.2.1, Fig. RD.6.2.1(b)

$A_{Vc0} = 4.5 c_{a1}^2$ ACI 318-08 Eq. (D-23)

$\Psi_{ec,V} = \left(\frac{1}{1 + \frac{2e_v}{3c_{a1}}}\right) \leq 1.0$ ACI 318-08 Eq. (D-26)

$\Psi_{ed,V} = 0.7 + 0.3\left(\frac{c_{a2}}{1.5c_{a1}}\right) \leq 1.0$ ACI 318-08 Eq. (D-28)

$\Psi_{h,V} = \sqrt{\frac{1.5c_{a1}}{h_a}} \geq 1.0$ ACI 318-08 Eq. (D-29)

$V_b = \left(7\left(\frac{l_e}{d_a}\right)^{0.2}\sqrt{d_a}\right)\lambda \sqrt{f'_c}\, c_{a1}^{1.5}$ ACI 318-08 Eq. (D-24)

Variables

c_{a1} [in.]	c_{a2} [in.]	e_{cV} [in.]	$\Psi_{c,V}$	h_a [in.]
4.000	-	0.000	1.400	8.000

l_e [in.]	λ	d_a [in.]	f'_c [psi]	$\Psi_{parallel,V}$
2.160	1.000	0.500	3000	1.000

Calculations

A_{Vc} [in.^{2}]	A_{Vc0} [in.^{2}]	$\Psi_{ec,V}$	$\Psi_{ed,V}$	$\Psi_{h,V}$	V_b [lb]
96.00	72.00	1.000	1.000	1.000	2906

Results

V_{cbg} [lb]	$\phi_{concrete}$	ϕV_{cbg} [lb]	V_{ua} [lb]
5425	0.700	3797	3636

5 Warnings

- Load re-distributions on the anchors due to elastic deformations of the anchor plate are not considered. The anchor plate is assumed to be sufficiently stiff, in order not to be deformed when subjected to the loading! Input data and results must be checked for agreement with the existing conditions and for plausibility!
- Condition A applies when supplementary reinforcement is used. The Φ factor is increased for non-steel Design Strengths except Pullout Strength and Pryout strength. Condition B applies when supplementary reinforcement is not used and for Pullout Strength and Pryout Strength. Refer to your local standard.
- Refer to the manufacturer's product literature for cleaning and installation instructions.
- Checking the transfer of loads into the base material and the shear resistance are required in accordance with ACI 318 or the relevant standard!

Fastening meets the design criteria!

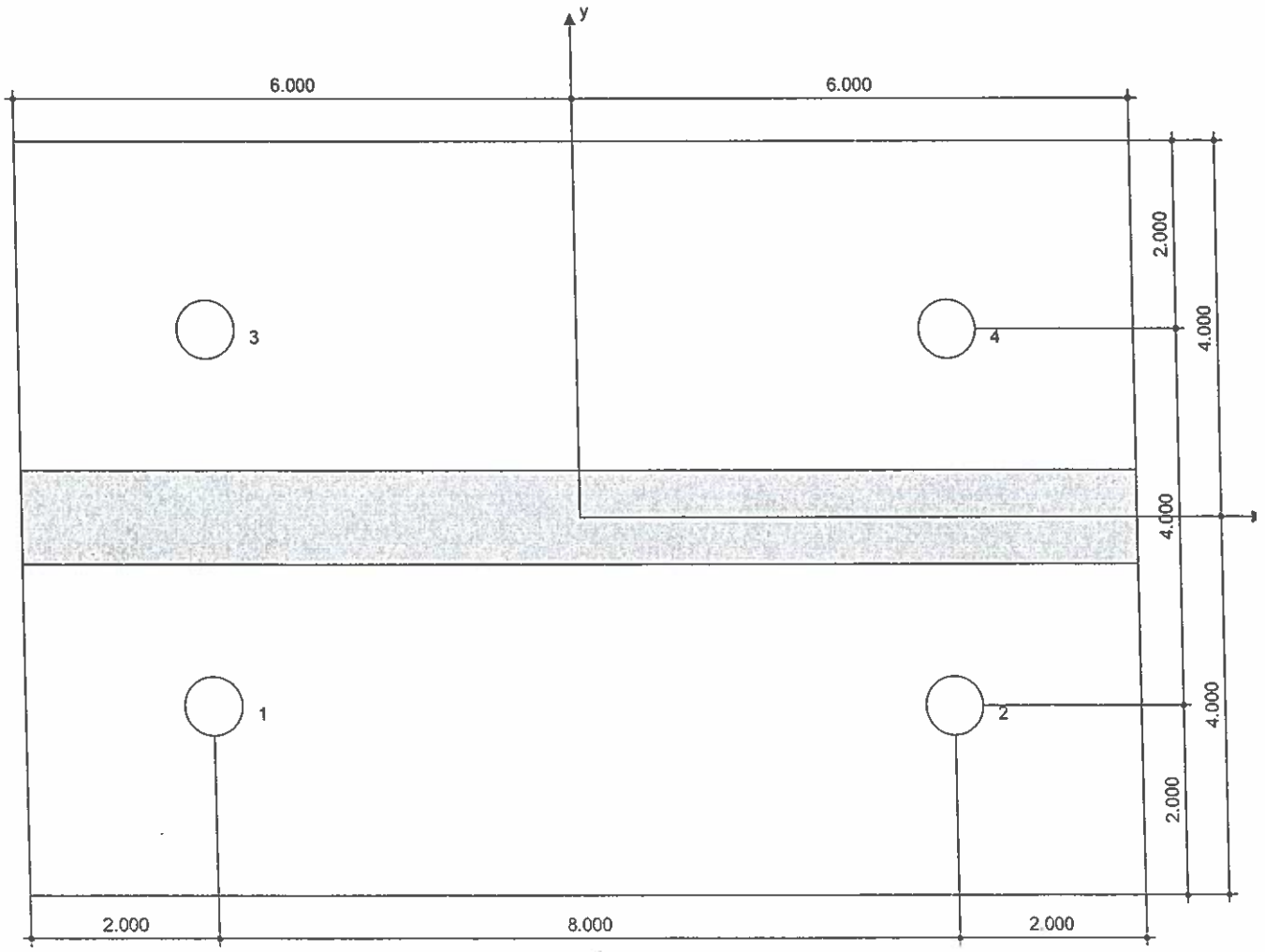
Company:
Specifier:
Address:
Phone I Fax:
E-Mail:

6 Installation data

Anchor plate, steel: -
Profile: Rectangular plates and bars (AISC); 12.000 x 1.000 x 0.000 in.
Hole diameter in the fixture: d_f = 0.625 in.
Plate thickness (input): 0.500 in.
Recommended plate thickness: not calculated
Drilling method: Hammer drilled
Anchor type and diameter: KWIK HUS-EZ (KH-EZ) 1/2 (3)
Installation torque: 540.001 in.lb
Hole diameter in the base material: 0.500 in.
Hole depth in the base material: 3.375 in.
Minimum thickness of the base material: 4.750 in.
Cleaning: Manual cleaning of the drilled hole according to instructions for use is required.

6.1 Recommended accessories

Drilling	Cleaning	Setting
• Suitable Rotary Hammer	• Manual blow-out pump	• Torque wrench
• Properly sized drill bit		



Coordinates Anchor in.

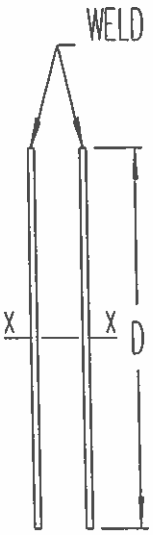
Anchor	x	y	c_{-x}	c_{+x}	c_{-y}	c_{+y}
1	-4.000	-2.000	6.000	12.000	-	-

7 Remarks; Your Cooperation Duties

- Any and all information and data contained in the Software concern solely the use of Hilti products and are based on the principles, formulas and security regulations in accordance with Hilti's technical directions and operating, mounting and assembly instructions, etc., that must be strictly complied with by the user. All figures contained therein are average figures, and therefore use-specific tests are to be conducted prior to using the relevant Hilti product. The results of the calculations carried out by means of the Software are based essentially on the data you put in. Therefore, you bear the sole responsibility for the absence of errors, the completeness and the relevance of the data to be put in by you. Moreover, you bear sole responsibility for having the results of the calculation checked and cleared by an expert, particularly with regard to compliance with applicable norms and permits, prior to using them for your specific facility. The Software serves only as an aid to interpret norms and permits without any guarantee as to the absence of errors, the correctness and the relevance of the results or suitability for a specific application.
- You must take all necessary and reasonable steps to prevent or limit damage caused by the Software. In particular, you must arrange for the regular backup of programs and data and, if applicable, carry out the updates of the Software offered by Hilti on a regular basis. If you do not use the AutoUpdate function of the Software, you must ensure that you are using the current and thus up-to-date version of the Software in each case by carrying out manual updates via the Hilti Website. Hilti will not be liable for consequences, such as the recovery of lost or damaged data or programs, arising from a culpable breach of duty by you.

Project Name:19-0207 _ 399 Golden Beach Trellis **Date:5/1/2019**

Weld Design For Aluminum Plate



Weld and Section Data:

$F_v := 7.00$ Ultimate Tensile Strength (ksi)

$d := 12.0$ Depth of Steel Plate (in)

Load Data:

$M_{weld} := 0.0$ Moment (Kips-ft)

$V_{weld} := 2.272$ Shear (Kips)

$P_{weld} := 0.00$ Tension or Compression (Kips)

Check Weld Section :

$F_v := F_v$ $F_v = 7.00$ ksi

$t_e :=$ $x \leftarrow 0.00001$

while $\sqrt{\left[\frac{P_{weld}}{2(d)\cdot x} + \frac{M_{weld}\cdot 12}{\left(\frac{d^2}{3}\right)\cdot x}\right]^2 + \left(\frac{V_{weld}}{2d\cdot x}\right)^2} \geq F_v$

$x \leftarrow x + 0.03125$

$t_e = 0.03$ in

$S_{weld} := \left(\frac{d^2}{3}\right)\cdot t_e$ $S_{weld} = 1.50$ in^3

Check Bending Stress :

$$f_b := \frac{M_{weld} \cdot (12)}{S_{weld}}$$

$f_b = 0.00$ ksi

Check Shear Stress :

$$f_v := \frac{2V_{weld}}{A_{weld}} + \frac{T_{weld} \cdot 12 \cdot r}{J_{weld}}$$

$f_v = 4.43$ ksi

Check Axial Stress :

$$f_a := \frac{P_{weld}}{A_{weld}}$$

$f_a = 0.00$ Ksi

Check Combined Stress :

$$f_{weld} := \sqrt{(f_b + f_a)^2 + f_v^2}$$

$f_{weld} = 4.43$ ksi

$$STRESS := \begin{cases} \text{"N.G."} \\ \text{"OK"} \quad \text{if } F_v \geq f_{weld} \end{cases}$$

STRESS = "OK"

$$W_{weld} := \begin{cases} \text{"3/16"} \quad \text{if } \frac{t_e}{0.707} \leq 0.1875 \\ \text{" 1/4"} \quad \text{if } 0.1875 < \frac{t_e}{0.707} \leq 0.25 \\ \text{"3/8"} \quad \text{if } 0.25 < \frac{t_e}{0.707} \leq 0.375 \\ \text{"1/2"} \quad \text{if } 0.375 < \frac{t_e}{0.707} \leq 0.50 \\ \text{"5/8"} \quad \text{if } 0.50 < \frac{t_e}{0.707} \leq 0.625 \\ \text{"3/4"} \quad \text{if } 0.625 < \frac{t_e}{0.707} \leq 0.750 \\ \text{"1"} \quad \text{if } 0.75 < \frac{t_e}{0.707} \leq 1.00 \end{cases}$$

$W_{weld} = \text{"3/16"}$

Town of Golden Beach
Building Regulatory Advisory Board
Hearing Date

JUL 09 2019

APPROVED_____
DISAPPROVED_____
VARIANCE REQ:_____

399 GOLDEN BEACH TRELLIS

399 GOLDEN BEACH DR GOLDEN
BEACH, FL 33160-2225

TRELLIS

INDEX OF DRAWINGS

SHEET	DESCRIPTION
SD-0.0	COVER PAGE.
SD-0.1	GENERAL NOTES.
SD-0.2	GENERAL NOTES.
SD-1.0	KEY PLAN
SD-2.0	ALUMINUM TRELLIS PLAN VIEW AND SECTION.
SD-2.1	DETAILS
SD-2.2	DETAILS AND CONNECTION

Eastern
Engineering Group
3401 NW 82nd Avenue
Suite 370
Miami, FL 33122
(305) 599-8133
info@easterneg.com
www.easterneg.com

NO.	DESCRIPTION	DATE	APP'D

DATE : 5/14/19
PM : GONZALO A. PAZ, P.E.
DRAWN BY : M.M.R
DESIGNED BY : M.M.R
CHECKED BY : G.P.
JOB NO. : 19-0207
SCALE : AS SHOWN

399 GOLDEN BEACH TRELLIS

COVER PAGE

TYPE OF PROJECT: TRELLIS
PROJECT LOCATION: 399 GOLDEN BEACH DR GOLDEN BEACH, FL 33160-2225
CLIENT: JOSES ORNAMENTAL WELDING

GONZALO A. PAZ
LICENSED
No 60734
STATE OF FLORIDA
PROFESSIONAL ENGINEER
5/14/2019

SHEET
SD-0.0

GENERAL NOTES:

1. ALL WORK SHALL CONFORM TO FLORIDA BUILDING CODE 2017.
2. IT IS THE INTENT OF THESE DRAWINGS TO BE IN ACCORDANCE WITH APPLICABLE CODES AND AUTHORITIES HAVING JURISDICTION. ANY DISCREPANCIES BETWEEN THESE DRAWINGS AND APPLICABLE CODES SHALL BE IMMEDIATELY BROUGHT TO THE ATTENTION OF THE ENGINEER.
3. EXISTING UTILITIES SHOWN ARE BASED ON INFORMATION SUPPLIED BY OTHERS. IT SHALL BE THE CONTRACTORS RESPONSIBILITY TO MEET WITH ALL APPLICABLE UTILITY COMPANIES TO VERIFY ALL UNDER– GROUND FACILITIES PRIOR TO THE BEGINNING OF CONSTRUCTION.
   ALL EXCAVATIONS SHALL PROCEED WITH EXTREME CAUTION AT ALL TIMES. IN THE EVENT THAT EXISTING UTILITIES ARE DAMAGED, IT SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR TO REPAIR OR REPLACE ALL DAMAGES.
4. THIS WORK REQUIRES A BUILDING PERMIT. DO NOT BEGIN WORKING UNTIL A BUILDING PERMIT IS OBTAINED.
5. CONTRACTOR IS TO FURNISH ALL LABOR, MATERIALS, SERVICES AND EQUIPMENT NECESSARY TO COMPLETE ALL WORK SHOWN ON THE DRAWINGS AND SPECIFIED HEREIN.
6. DO NOT SCALE DRAWINGS, DIMENSIONS GOVERN.
7. ENGINEER'S VISITS TO THE SITE, AS PER G.C OR OWNER'S REQUEST DURING CONSTRUCTION SHALL BE SCHEDULED WITHIN 24 HOURS PRIOR TO INSPECTION.
8. THE CONTRACTOR SHALL MAKE REQUIRED ARRANGEMENTS, SECURE AND PAY FOR ALL BARRICADES, ENCLOSURES, AND FENCING AS NEEDED FOR AND DURING THE PROGRESS TO PROTECT ADJACENT PROPERTIES.
9. THE CONTRACTOR SHALL NOT PROCEED WITH ANY ADDITIONAL SERVICES OR WORK WITHOUT PRIOR NOTIFICATION TO THE OWNER.
10. THE CONTRACTOR IS SOLELY RESPONSIBLE FOR MEANS AND METHODS CONSTRUCTION, AND FOR THE SEQUENCES AND PROCEDURES TO BE USED.
11. EXISTING GRADES WERE TAKEN FROM THE BEST AVAILABLE DATA AND MAY NOT ACCURATELY REFLECT PRESENT CONDITIONS. CONTRACTOR SHALL BE RESPONSIBLE FOR FAMILIARIZING WITH CURRENT SITE CONDITIONS, AND SHALL REPORT ANY DISCREPANCIES TO THE ENGINEER PRIOR TO STARTING WORK.
12. CONTRACTOR SHALL VERIFY ALL DIMENSIONS AND EXISTING CONDITIONS AT THE JOB SITE. ANY DISCREPANCIES BETWEEN PLANS, SECTIONS AND DETAILS OR THE APPLICABLE CODES OR REGULATIONS SHALL BE BROUGHT TO THE ATTENTION OF THE ARCHITECT OR ENGINEER DURING BIDDING OR BEFORE WORK BEGINS IN ORDER TO CLARIFY THE REQUIREMENTS AND TO EFFECT THE NECESSARY MODIFICATIONS, CHANGES AND /OR INSTRUCTIONS.
13. CONTRACTOR TO VERIFY THE LOCATION OF ANY EXISTING UTILITY LINE AND IMPROVEMENTS, AND SHALL BE RESPONSIBLE FOR REPAIRS FOR ANY DAMAGE AS A RESULT OF THE WORK.
14. CONTRACTOR SHALL BE RESPONSIBLE FOR RESETTING ALL DISTURBED EXISTING CONDITIONS AND PROPER DISPOSAL OF ANY EXTRA MATERIALS & GARBAGE FROM THE SITE AFTER COMPLETION OF WORK.
15. DRAWINGS AND DIMENSIONS ARE BASED UPON DRAWINGS SUPPLIED BY THE CLIENT. EASTERN ENGINEERING GROUP WILL NOT BE RESPONSIBLE FOR ERRORS OR MISINTERPRETATIONS OF THE SYSTEM DESIGNED BY US BASED ON CLIENT CONFIRMED DESIGN AND DIMENSIONS. ADDITIONAL DRAFTING TIME EMPLOYED IN THE CHANGE OF THE DESIGN AFTER SIGNING AND SEALING OF DRAWINGS WILL RESULT IN ADDITIONAL COST.
16. DO NOT SUBSTITUTE MATERIALS, EQUIPMENTS OR METHODS OF CONSTRUCTION UNLESS SUCH SUBSTITUTIONS OR CHANGES HAVE BEEN APPROVED IN WRITING BY THE OWNER.
17. EASTERN ENGINEERING GROUP HAS GENERATED THESE SHOP DRAWINGS BASED ON A PROVIDED DESIGN THAT HAS BEEN DEVELOPED BY A LICENSED ARCHITECT OR A COMPETENT LICENSED DESIGN PROFESSIONAL WHO CONFIRMED COMPLIANCE WITH ALL APPLICABLE NATIONAL AND FLORIDA BUILDING CODES.
18. EASTERN ENGINEERING GROUP SHALL NOT BE LEGALLY RESPONSIBLE FOR THE STRUCTURAL DESIGN OF ANY COMPONENT AND/OR PRODUCT USED IN OUR PROJECTS WHICH HAS BEEN PREVIOUSLY GRANTED A PATENT OR COPYRIGHT. THE CONFIRMATION OF INTELLECTUAL PROPERTY OWNERSHIP IS BEYOND OUR SCOPE AS STRUCTURAL ENGINEERS AND SHALL BE THE SOLE RESPONSIBILITY OF OUR CLIENT.
19. EASTERN ENGINEERING GROUP HAS EXCLUSIVELY DESIGNED THE STRUCTURE AND/OR BUILDING COMPONENTS IN COMPLIANCE WITH THE APPLICABLE EDITION OF THE FLORIDA BUILDING CODE AND DESIGN STANDARDS FOR STRUCTURAL REQUIREMENTS ONLY. THE EXISTING STRUCTURE MUST SUPPORT THE LOADS IMPOSED BY THE SYSTEM OR SYSTEMS. ENGINEER ON RECORD OF THE BUILDING OR CERTIFIED PROFESSIONAL ENGINEER SHALL VERIFY THE STRUCTURE FOR SUCH LOADINGS.
20. ELEMENTS WILL BE AS DESIGNED BY EASTERN ENGINEERING GROUP AND AS APPROVED BY ARCHITECT AND/OR OWNERS, TO CONFORM GENERALLY WITH THE ARCHITECTURAL DRAWING AND SPECIFICATIONS.
21. FLORIDA BUILDING CODE, 2017 EDITION LOADS:

ALUMINUM TRELLIS:
DL=20 PSF
LL=20 PSF
WIND UP=77.06 PSF

WIND LOAD AS PER ASCE 7–10

KD=0.85, CATEGORY 2

WIND VELOCITY AS PER ASCE 7–10

	MIAMI–DADE
CATEGORY 2	---------- 175 mph

22. THE EXISTING STRUCTURE MUST SUPPORT THE LOADS IMPOSED BY THE SYSTEM OR SYSTEMS. ENGINEER ON RECORD OF THE BUILDING OR FLORIDA PROFESSIONAL ENGINEER SHALL VERIFY THE STRUCTURE FOR SUCH LOADINGS. THIS SHOP DRAWINGS DOES NOT COVER THE EXISTING STRUCTURE.
23. THE QUANTITIES AND DIMENSIONS SHOWN ON THE DRAWINGS ARE BASED ON THE ARCHITECTURAL DRAWINGS.
24. ALL DIMENSIONS TO BE SITE VERIFIED.

Eastern
Engineering Group
3401 NW 82nd Avenue
Suite 370
Miami, FL 33122
(305) 599-6133
info@easterneg.com
www.easterneg.com

NO.	DESCRIPTION	DATE	APP'D

DATE : 9/14/18
PM : GONZALO A. PAZ, P.E.
DRAWN BY : M.M.R
DESIGNED BY : M.M.R
CHECKED BY : G.P.
JOB NO. : 19-0207
SCALE : AS SHOWN

399 GOLDEN BEACH TRELLIS

GENERAL NOTES
TYPE OF PROJECT: TRELLIS
PROJECT LOCATION: 399 GOLDEN BEACH DR GOLDEN BEACH, FL. 33160-2225
CLIENT: JOSES CINAMISTAL WELDING

GONZALO A. PAZ
LICENSE
No 60734
STATE OF FLORIDA
PROFESSIONAL ENGINEER

SD-0.1

GLASS

1. ALL GLASS IN THIS PROJECT WILL BE SAFETY GLASS ACCORDING TO FBC 2017–SECTION 2407.1

2. LAMINATED GLASS 3/4" NOMINAL. COMPOSED OF (2) 3/8" FULLY TEMPERED GLASS WITH 1/16 SENTRY GLASS PLUS INTERLAYER FILM.

3. FULLY TEMPERED GLASS AND LAMINATED GLASS SHALL COMPLY WITH CATEGORY II OF CPSC 16 CFR 1201 OR CLASS A OF ANSI Z97.1, LISTED IN CHAPTER 35.ANSI Z97.1.

ALUMINUM

1. ALUMINUM SHALL MEET THE FOLLOWING REQUIREMENTS UNLESS NOTED OTHERWISE ON THE DRAWINGS:

TYPE	Fb	Fb(WELDED)	Fv	Fv(WELDED)
6061–T6(TUBES & SHAPES)	19.0 KSI	9.0 KSI	12.0 KSI	5.0 KSI
6061–T6(ROUND & OVAL)	24.0 KSI	10.5 KSI	12.0 KSI	5.0 KSI

2. WELDING: ALUMINUM ALLOY 5356. CLEANING: SSPC–SP2 "HAND TOOL CLEANING".

3. PAINT ALUMINUM AND STEEL HOT GALVANIZED SURFACES IN CONTACT WITH CONCRETE WITH ALKALI–RESISTANT COATINGS, SUCH AS HEAVY–BODIED BITUMINOUS PAINT OR WATER–WHITE METHACRYLATE LACQUER.

4. ISOLATE DISSIMILAR MATERIALS WITH ALKALI–RESISTANT COATINGS, SUCH AS HEAVY–BODIED BITUMINOUS PAINT OR WATER–WHITE METHACRYLATE LACQUER.

Eastern
Engineering Group
3401 NW 82nd Avenue
Suite 370
Miami, FL 33122
(305) 599-8133
info@easterneg.com
www.easterneg.com

NO.	DESCRIPTION	DATE	APP'D

DATE : 5/14/19
PM : GONZALO A. PAZ, P.E.
DRAWN BY : M.M.R
DESIGNED BY : M.M.R
CHECKED BY : G.P.
JOB NO. 119–0207
SCALE : AS SHOWN

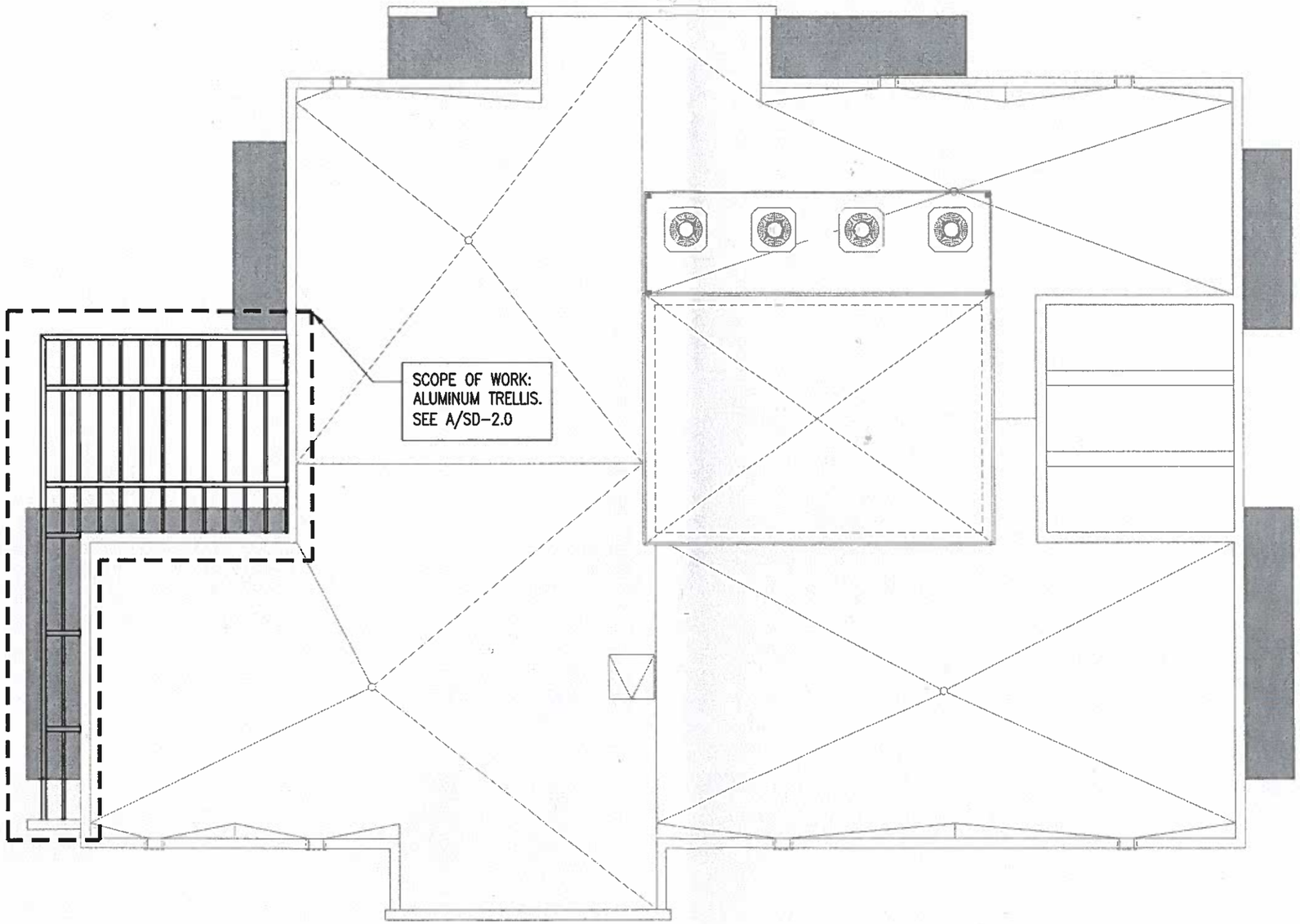
399 GOLDEN BEACH TRELLIS

GENERAL NOTES

TYPE OF PROJECT: TRELLIS
PROJECT LOCATION: 399 GOLDEN BEACH DR GOLDEN BEACH, FL. 33160–2225
CLIENT: JOSES ORNAMENTAL WELDING

GONZALO A. PAZ
LICENSED
No 6
STATE OF FLORIDA
PROFESSIONAL ENGINEER

SD-0.2



A / SD-1.0 **ROOF KEY PLAN.**

SCALE: 1/8"=1'-0"

Eastern
Engineering Group

3401 NW 82nd Avenue
Suite 370
Miami, FL 33122

(305) 599-8133

info@easterneg.com

www.easterneg.com

NO.	DESCRIPTION	DATE	APP'D

DATE : 5/14/19
PM : GONZALO A. PAZ, P.E.
DRAWN BY : M.M.R
DESIGNED BY : M.M.R
CHECKED BY : G.P.
JOB NO. : 19-0207
SCALE : AS SHOWN

399 GOLDEN BEACH TRELLIS

KEY PLAN

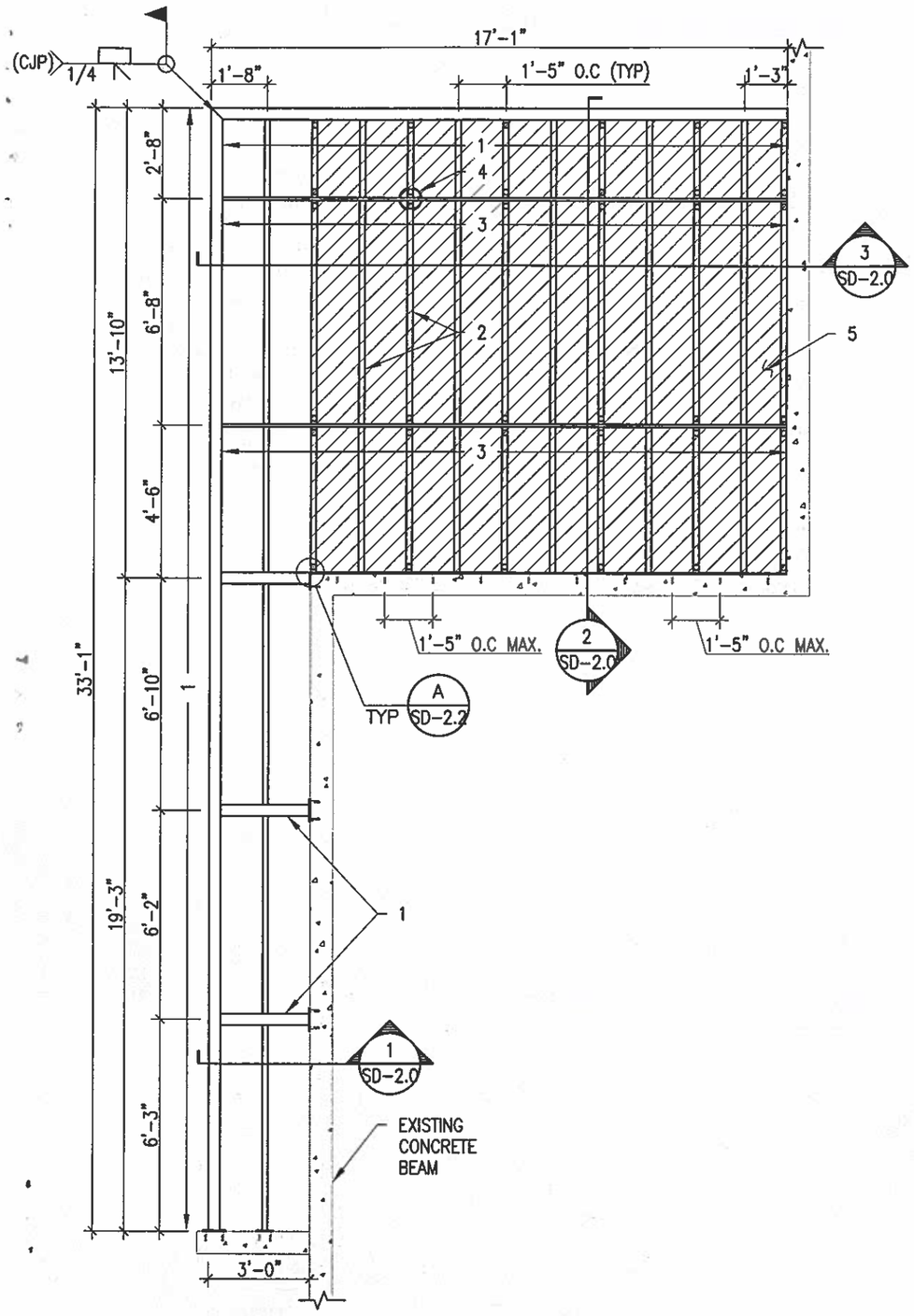
TYPE OF PROJECT: TRELLIS

PROJECT LOCATION: 399 GOLDEN BEACH DR, GOLDEN BEACH, FL. 33160-2225

CLIENT: JOSES ORNAMENTAL WELDING

GONZALO A. PAZ
LICENSE
No 60733
STATE OF FLORIDA
PROFESSIONAL ENGINEER
5/22/2019

SD-1.0



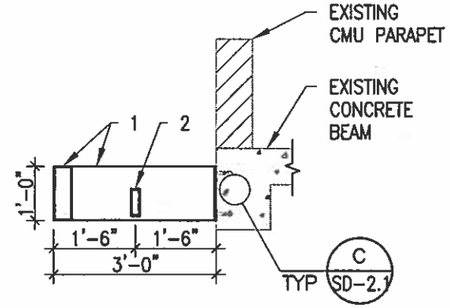
A ALUMINUM TRELLIS PLAN VIEW
SD-2.0 SCALE: 1/4"=1'-0"

LEGEND:

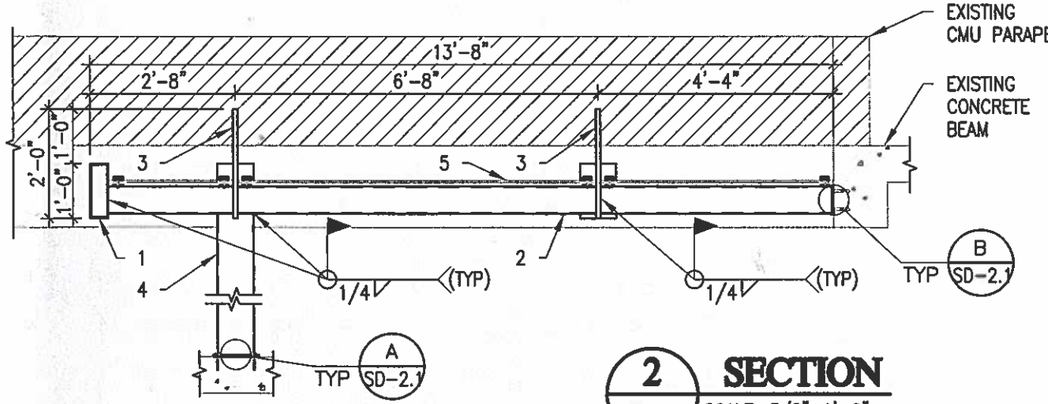
1. 12"X4"X¼" THK. ALUMINUM TUBE, ALLOY 6061-T6
2. 6"X2"X¼" THK. ALUMINUM TUBE, ALLOY 6061-T6
3. 1" THK. ALUMINUM PLATE, ALLOY 6061-T6
4. ø8"X¼" THK. ALUMINUM ROUND COLUMN (22' HEIGHT MAX.) , ALLOY 6061-T6
5. ¾" THK. LAMINATED TEMPERED GLASS, SEE GENERAL NOTES FOR GLASS SPECS.
6. ø2" STANDOFF PAIR @ 2'-10" O.C MAX, MIN (4) PER GLASS PANEL
7. 6"X⅜" THK. ALUMINUM CONTINUOUS PLATE, ALLOY 6061-T6
8. ø½" WEDGE BOLT BY POWER FASTENER OR EQUAL W/ 3" MIN. EMBED, 6" MIN EDGE DIST. & 4" MIN SPACING.
9. 8"X12"X⅜" THK. ALUMINUM PLATE, ALLOY 6061-T6
10. 10"X10"X½" THK. ALUMINUM PLATE, ALLOY 6061-T6
11. ø½" S.S WEDGE BOLT BY HILTI OR EQUAL APPROVED W/ 3" MIN. EMBEDMENT INTO CONC., W/ 8" MIN. SPACING, W/ 12" MIN. EDGE DISTANCE.

NOTE:

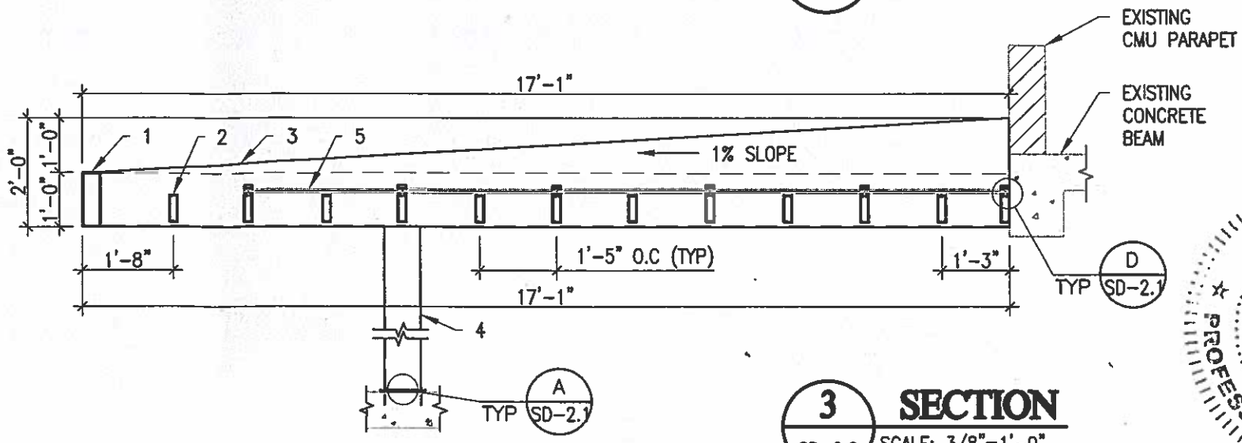
DIMENSION MUST BE VERIFY ON FIELD



1 SECTION
SD-2.0 SCALE: 3/8"=1'-0"



2 SECTION
SD-2.0 SCALE: 3/8"=1'-0"



3 SECTION
SD-2.0 SCALE: 3/8"=1'-0"

Eastern
Engineering Group
3401 NW 82nd Avenue
Suite 370
Miami, FL 33122
(305) 599-8133
Info@easterneg.com
www.easterneg.com

NO.	DESCRIPTION	DATE	APP'D

DATE : 5/14/18
PM : GONZALO A. PAZ, P.E.
DRAWN BY : M.M.R
DESIGNED BY : M.M.R
CHECKED BY : G.P.
JOB NO. : 18-2207
SCALE : AS SHOWN

399 GOLDEN BEACH TRELLIS

ALUMINUM TRELLIS PLAN VIEW AND SECTION

TYPE OF PROJECT: TRELLIS
PROJECT LOCATION: 399 GOLDEN BEACH DR GOLDEN BEACH, FL 33160-2225
CLIENT: JOSES ORNAMENTAL WELDING

GONZALO A. PAZ
LICENSE NO. 79279
STATE OF FLORIDA
PROFESSIONAL ENGINEER

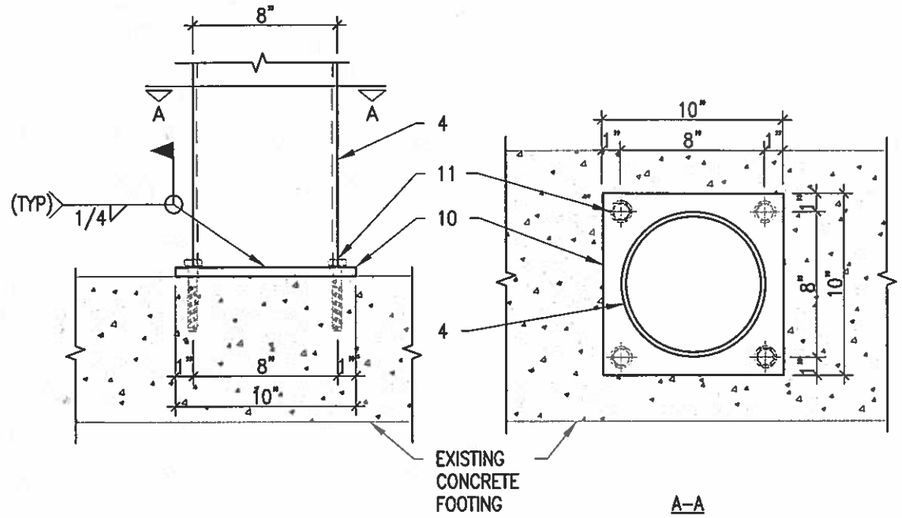
SD-2.0

LEGEND:

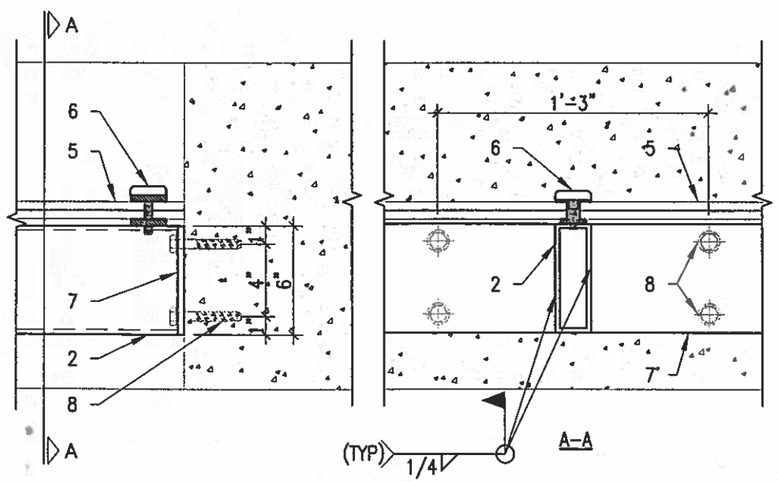
1. 12"X4"X¼" THK. ALUMINUM TUBE, ALLOY 6061-T6
2. 6"X2"X¼" THK. ALUMINUM TUBE, ALLOY 6061-T6
3. 1" THK. ALUMINUM PLATE, ALLOY 6061-T6
4. ø8"X¼" THK. ALUMINUM ROUND COLUMN (22' HEIGHT MAX.) , ALLOY 6061-T6
5. ¾" THK. LAMINATED TEMPERED GLASS, SEE GENERAL NOTES FOR GLASS SPECS.
6. ø2" STANDOFF PAIR @ 2'-10" O.C MAX, MIN (4) PER GLASS PANEL
7. 6"X⅜" THK. ALUMINUM CONTINUOUS PLATE, ALLOY 6061-T6
8. ø½" WEDGE BOLT BY POWER FASTENER OR EQUAL W/ 3" MIN. EMBED, 6" MIN EDGE DIST. & 4" MIN SPACING.
9. 8"X12"X⅜" THK. ALUMINUM PLATE, ALLOY 6061-T6
10. 10"X10"X½" THK. ALUMINUM PLATE, ALLOY 6061-T6
11. ø½" S.S WEDGE BOLT BY HILTI OR EQUAL APPROVED W/ 3" MIN. EMBEDMENT INTO CONC., W/ 8" MIN. SPACING, W/ 12" MIN. EDGE DISTANCE.

NOTE:

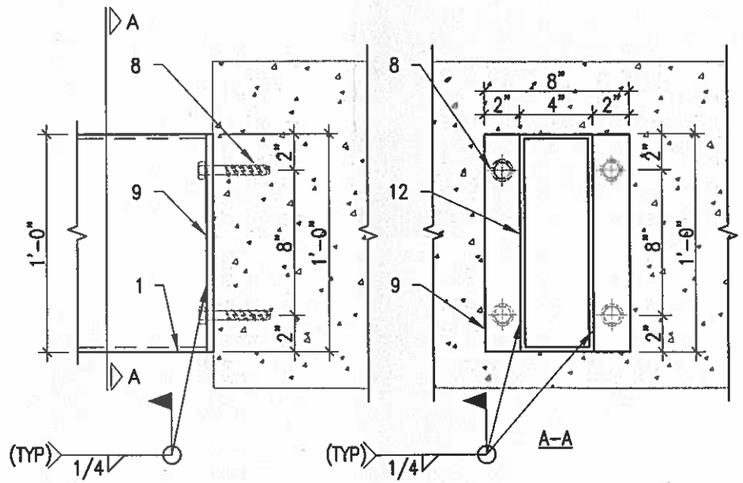
DIMENSION MUST BE VERIFY ON FIELD



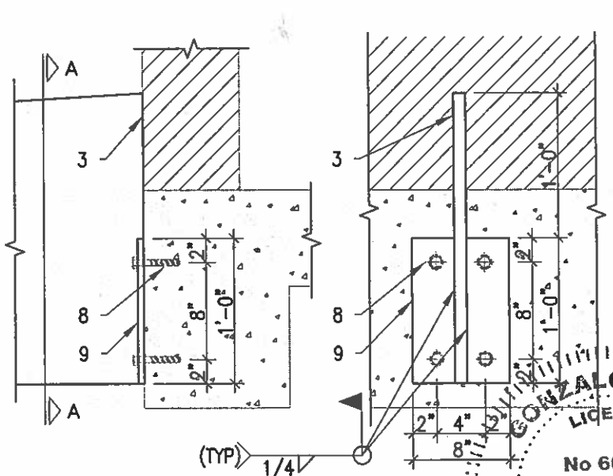
A DETAIL
SD-2.1 SCALE: 1 1/2"=1'-0"



B DETAIL
SD-2.1 SCALE: 1 1/2"=1'-0"



C DETAIL
SD-2.1 SCALE: 1 1/2"=1'-0"



D DETAILS
SD-2.1 SCALE: 1"=1'-0"

Eastern
Engineering Group
3401 NW 82nd Avenue
Suite 370
Miami, FL 33122
(305) 599-8133
info@easterneg.com
www.easterneg.com

NO.	DESCRIPTION	DATE	APP'D

DATE : 5/14/19
PM : GONZALO A. PAZ, P.E.
DRAWN BY : MMR
DESIGNED BY : MMR
CHECKED BY : G.P.
JOB NO. : 19-2007
SCALE : AS SHOWN

399 GOLDEN BEACH TRELLIS

TYPE OF PROJECT: TRELLIS
PROJECT LOCATION: 399 GOLDEN BEACH DR GOLDEN BEACH, FL 33160-2325
CLIENT: JOSES ORNAMENTAL WELDING

DETAILS

GONZALO A. PAZ
LICENSE
No 60734
STATE OF
FLORIDA
PROFESSIONAL ENGINEER

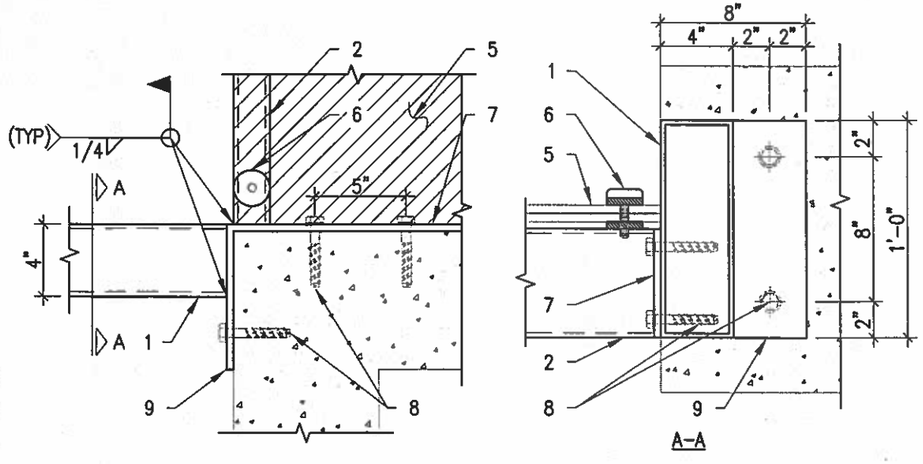
SHEET
SD-2.1

LEGEND:

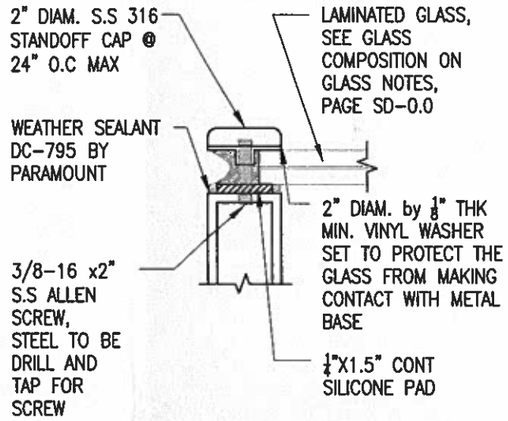
1. 12"X4"X¼" THK. ALUMINUM TUBE, ALLOY 6061-T6
2. 6"X2"X¼" THK. ALUMINUM TUBE, ALLOY 6061-T6
3. 1" THK. ALUMINUM PLATE, ALLOY 6061-T6
4. ø8"X¼" THK. ALUMINUM ROUND COLUMN (22' HEIGHT MAX.) , ALLOY 6061-T6
5. ¾" THK. LAMINATED TEMPERED GLASS, SEE GENERAL NOTES FOR GLASS SPECS.
6. ø2" STANDOFF PAIR @ 2'-10" O.C MAX, MIN (4) PER GLASS PANEL
7. 6"X⅜" THK. ALUMINUM CONTINUOUS PLATE, ALLOY 6061-T6
8. ø½" WEDGE BOLT BY POWER FASTENER OR EQUAL W/ 3" MIN. EMBED, 6" MIN EDGE DIST. & 4" MIN SPACING.
9. 8"X12"X⅜" THK. ALUMINUM PLATE, ALLOY 6061-T6
10. 10"X10"X½" THK. ALUMINUM PLATE, ALLOY 6061-T6
11. ø½" S.S WEDGE BOLT BY HILTI OR EQUAL APPROVED W/ 3" MIN. EMBEDMENT INTO CONC., W/ 8" MIN. SPACING, W/ 12" MIN. EDGE DISTANCE.

NOTE:

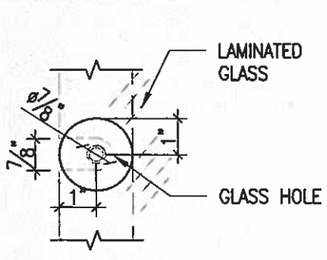
DIMENSION MUST BE VERIFY ON FIELD



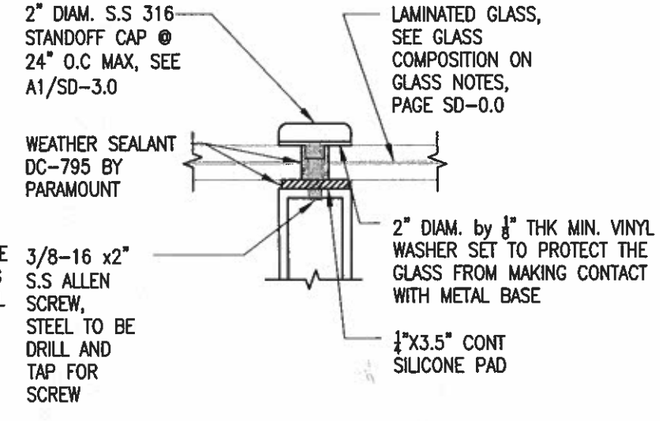
A DETAIL
SD-2.2 SCALE: 1 1/2"=1'-0"



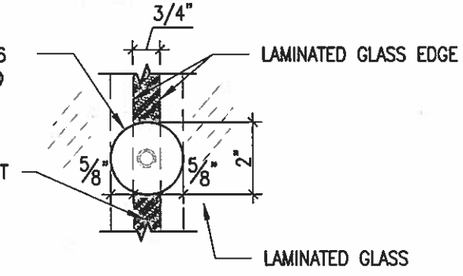
EDGE CONNECTION DETAIL-SECTION



EDGE CONNECTION DETAIL-PLAN VIEW

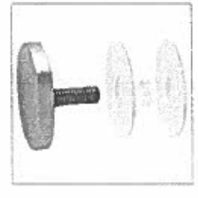


INTERMEDIATE CONNECTION DETAIL-SECTION



INTERMEDIATE CONNECTION DETAIL-PLAN VIEW

APPLY LOCTITE OVER ALLEN SCREWS PRIOR TO INSTALLING STANDOFF CAPS. (TO PREVENT CAPS FROM LOOSENING).



B GLASS CONNECTIONS
SD-2.2 SCALE: 3"=1'-0"

Eastern
Engineering Group
3401 NW 82nd Avenue
Suite 370
Miami, FL 33122
(305) 599-8133
info@easterneg.com
www.easterneg.com

NO.	DESCRIPTION	DATE	APP'D

DATE : 5/14/19
PM : GONZALO A. PAZ, P.E.
DRAWN BY : M.M.R
DESIGNED BY : M.M.R
CHECKED BY : G.P.
JOB NO. : 19-0207
SCALE : AS SHOWN

399 GOLDEN BEACH TRELLIS

DETAILS AND CONNECTION

TYPE OF PROJECT: TRELLIS
PROJECT LOCATION: 399 GOLDEN BEACH DR GOLDEN BEACH, FL 33160-2235
CLIENT: JOSES ORNAMENTAL WELDING

GONZALO A. PAZ LICENSE No 60734 STATE OF FLORIDA PROFESSIONAL ENGINEER 5/14/2019

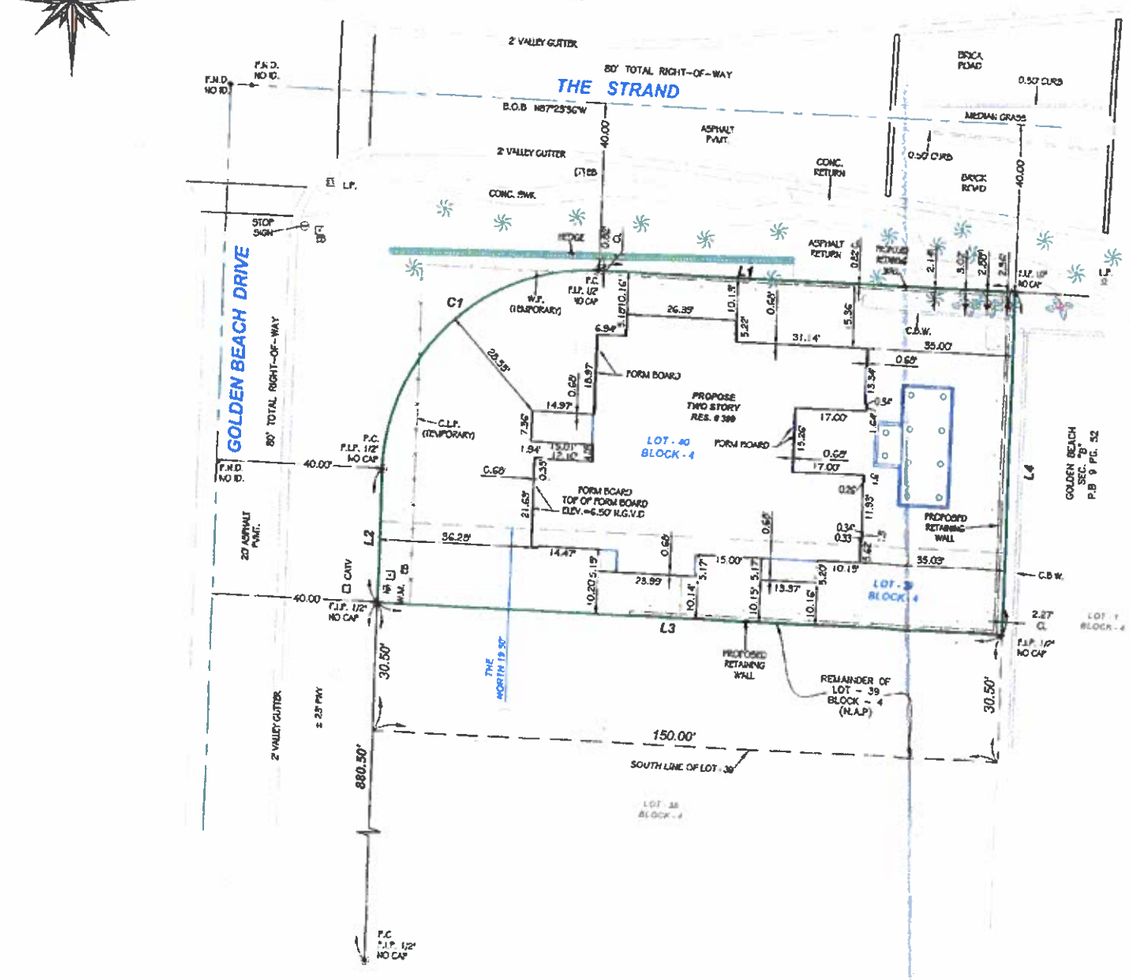
SHEET SD-2.2

JOHN IBARRA & ASSOCIATES, INC.
Professional Land Surveyors & Mappers
WWW.IBARRALANDSURVEYORS.COM
777 N.W. 72nd AVENUE
SUITE 3026
MIAMI, FLORIDA 33126
PH (305) 592-0400
FAX (305) 592-0401
4001 TAMIAMI TRAIL NORTH
SUITE J 205
NAPLES, FL 34103
PH (239) 640-3680
FAX (239) 640-2664

AS-BUILT SURVEY

CURVE TABLE						
CURVE	LENGTH	RADIUS	DELTA	CH. B.	CH. L.	T
C1	78.54'	50.00'	90°00'	S47°36'24"W	70.71'	50.00'

LINE TABLE		
LINE	BEARING	LENGTH
L1	N87°23'36"W	100.00'
L2	S02°36'24"W	32.00'
L3	S87°23'36"E	150.00'
L4	N02°36'24"E	82.00'



GRAPHIC SCALE
-20 0 10 20 40 80
(IN FEET)

LEGAL DESCRIPTION:
NORTH 10.50' OF LOT 39 AND ALL OF LOT 40, BLOCK 4, GOLDEN BEACH SECTION E, ACCORDING TO THE PLAT THEREOF AS RECORDED IN PLAT BOOK 6, PAGE 122, OF THE PUBLIC RECORDS OF MIAMI-DADE COUNTY, FLORIDA.

PROPERTY ADDRESS:
399 GOLDEN BEACH DRIVE
GOLDEN BEACH, FL 33160

CERTIFICATION:
GALSKY CONSTRUCTION ENTERPRISES 2, LLC

ABBREVIATIONS

A	= ARC
A/C	= AIR CONDITIONER/ING
A.E.	= ACCESS EASEMENT
A.R.	= ALUMINUM ROOF
A.S.	= ALUMINUM SHED
ASPH.	= ASPHALT
B.C.	= BEGIN CURVE
B.M.	= BENCH MARK
B.M.	= BEGIN METAL
B.R.	= BUILDING
B.S.	= BEGIN SPIRAL
B.S.E.	= BROWARD COUNTY RECORDS
B.O.B.	= BASIS OF BEARINGS
B.S.L.	= BUILDING SETBACK LINE
CB	= CHLORIDE
C.B.	= CATCH BASIN
C.B.S.	= CONCRETE BLOCK STRUCTURE
C.B.W.	= CONCRETE BLOCK WALL
CH.	= CHORD
CH.B.	= CHORD BEARING
CH.L.	= CHORD LENGTH
C.	= CLEAN
C.O.	= CLEAN OUT
C.L.F.	= CHAIN LINK FENCE
C.M.E.	= CANAL MAINTENANCE EASEMENT
CONC.	= CONCRETE
C.P.P.	= CONCRETE UTILITY POLE
C.P.	= CONCRETE PORCH
D.E.	= DRAINAGE EASE
C.W.	= CONCRETE WALK
D.E.	= DRAINAGE EASEMENT
EASE.	= EASEMENT
ENTR.	= ENTRANCE
	= ADDRESS
ELEV.	= ELEVATION
E.T.P.	= ELECTRIC TRANSFORMER PAD
ELEC.	= ELEVATION
	= FIELD MEASURED
F.D.	= FOUND DISK
F.I.R.	= FOUND IRON PIPE
F.I.R.	= FOUND IRON ROD
F.F.E.	= FINISHED FLOOR ELEVATION
F.C.M.	= FOUND CONC. MON. & DISC
FT.	= FEET
FWC	= FEDERAL WATCHES, SEISMOGRAPHIC PROGRAM
F.N.	= FOUND NAIL
G	= GRID (OR GUTTER)
G.M.E.	= GRADING AND DRAINAGE EASEMENT
GR.E.	= GRADING EASEMENT
GR.	= GARAGE
G.F.F.	= GARAGE FINISH FLOOR
GF	= IRON FENCE
I.E.	= LEGIBLE EASEMENT
I.P.	= LIGHT POLE
L.F.E.	= LOWEST FLOOR ELEVATION
L.M.E.	= LAKE MAINTENANCE EASEMENT
M	= MEASURED
MH.	= MANHOLE
MEAS.	= MEASURED
M.D.C.	= MIAMI DADE COUNTY RECORDS
M.E.	= MAINTENANCE EASEMENT
MIS.	= MISCELLANEOUS
M.P.	= MONUMENT
M.R.B.	= MONUMENT RECORDED VERTICAL DATUM
N.G.V.D.	= NOT TO SCALE
N.T.S.	= NUMBER
NO.	= OVERHEAD
OH	= ENCROACHMENT
O.R.	= ORIGINAL UTILITY LINES
O.U.L.	= OFFICIAL RECORDS BOOK
O.R.B.	= OVERHANG
O.H.	= PERIMETER
PERIM.	= PLANTER
PL.	= PROPERTY LINE
Pl.	= POINT OF CONTROL CURVATURE
P.C.C.	= POINT OF CURVATURE
P.C.	= POINT OF TANGENCY
P.T.	= POINT OF COMPOUND CURVE
P.C.C.	= POINT OF COMMENCEMENT
P.O.C.	= POINT OF BEGINNING
P.O.B.	= POINT OF REVERSE CURVATURE
P.R.C.	= PARKING
PKG.	= PERMANENT REFERENCE MONUMENT
P.R.M.	= PROFESSIONAL LAND SURVEYOR
P.L.S.	= POWER POLE
P.P.	= POOL PUMP SLAB
P.P.S.	= PUBLIC UTILITY EASEMENT
P.U.E.	= RECORD (VARIABLE)
R	= RAD. RADIUS
RAD.	= RADIONICS
RAD.	= RADIO OR RODS
RAD.	= RADIUS OR RADIAL
RAD.	= RADIUS
R.S.P.	= ROOF SUPPORT PARTITIONS
REC.	= RECESS
STR.	= STORY
SWK.	= SIDEWALK
S.P.	= SET IRON PIPE
S	= SOUTH
S.P.	= SCREENED PORCH
S.V.	= SERVICE VALVE
	= SECONDS
T	= TANGENT
TB	= TEE (PLUMB) JOINT
T.B.M.	= TEMPORARY BENCHMARK
T.S.E.	= TELEVISION UTILITY EASEMENT
TSE	= TRAFFIC SIGNAL STOP
T.S.P.	= TRAFFIC SIGNAL POLE
TYP.	= TYPICAL
UE	= UTILITY
U.E.	= UTILITY EASEMENT
U.	= UTILITY POLE
WM	= WATER METER
WF	= WOOD FENCE
WP	= WOOD PORCH
WR	= WOOD ROOF
WV	= WATER VALVE
	= WORKABLE LINE
	= CENTER LINE
	= BOUNDARY LINE
	= BOUNDARY LINE
	= SET TB

SURVEYOR'S NOTES:
1. IF SHOWN, BEARINGS ARE REFERRED TO AN ASSUMED MERIDIAN, BY SAID PLAT IN THE DESCRIPTION OF THE PROPERTY, IF NOT, BEARINGS ARE THEN REFERRED TO COUNTY, TOWNSHIP MAPS.
2. THE CLOSURE IN THE BOUNDARY SURVEY IS ABOVE 1:7500 FT.
3. CERTIFICATE OF AUTHORIZATION LB # 7826.
4. ALL ELEVATIONS SHOWN ARE REFERRED TO NATIONAL GEODETIC VERTICAL DATUM OF 1929 MIAMI-DADE COUNTY BENCH MARK # F-344-R; LOCATOR NO. 2220 SE; ELEVATION IS 4.35 FEET OF N.G.V.D. OF 1929.

LEGAL NOTES TO ACCOMPANY SKETCH OF SURVEY :
- THERE MAY BE EASEMENTS RECORDED IN THE PUBLIC RECORDS NOT SHOWN ON THIS SURVEY
- EXAMINATIONS OF THE ABSTRACT OF TITLE WILL HAVE TO BE MADE TO DETERMINE RECORDED INSTRUMENTS, IF ANY, AFFECTING THE PROPERTY.
- THIS SURVEY IS SUBJECT TO DEDICATIONS, LIMITATIONS, RESTRICTIONS, RESERVATIONS OR EASEMENTS OF RECORD.
- LEGAL DESCRIPTIONS PROVIDED BY CLIENT OR ATTESTING TITLE COMPANY.
- BOUNDARY SURVEY MEANS A DRAWING AND/OR A GRAPHIC REPRESENTATION OF THE SURVEY WORK PERFORMED IN THE FIELD, COULD BE DRAWN AT A SHOWN SCALE AND/OR NOT TO SCALE. THE WALLS OR FENCES MAY BE EXAGGERATED FOR CLARITY PURPOSES.
- EASEMENTS AS SHOWN ARE PER PLAT BOOK, UNLESS DEPICTED OTHERWISE.
- THE TERM "ENCROACHMENT" MEANS VISIBLE AND ABOVE GROUND ENCROACHMENTS.
- ARCHITECTS SHALL VERIFY ZONING REGULATIONS, RESTRICTIONS, SETBACKS AND WILL BE RESPONSIBLE FOR SUBMITTING PLOT PLANS WITH CORRECT INFORMATION FOR "APPROVAL FOR AUTHORIZATION" TO THE PROPER AUTHORITIES IN NEW CONSTRUCTION.
- UNLESS OTHERWISE NOTED, THIS FIRM HAS NOT ATTEMPTED TO LOCATE FOOTING AND/OR FOUNDATIONS.
- FENCE OWNERSHIP NOT DETERMINED.
- THIS PLAN OF SURVEY, HAS BEEN PREPARED FOR THE EXCLUSIVE USE OF THE ENTITIES NAMED HEREON, THE CERTIFICATE DOES NOT EXTEND TO ANY UNNAMED PARTY.

FLOOD ZONE INFORMATION:
THE NFIP FLOOD MAPS HAVE DESIGNATED THE HEREIN DESCRIBED LAND TO BE SITUATED IN:
FLOOD ZONE: "AE"
BASE FLOOD ELEVATION: 6 FEET.
COMMUNITY: 120678
PANEL: 0453
SUFFIX: L
DATE OF FIRM: 09/11/2009
THE SUBJECT PROPERTY DOES LIE IN A SPECIAL FLOOD HAZARD AREA.

SURVEYOR'S CERTIFICATION:
I HEREBY CERTIFY THAT THIS BOUNDARY SURVEY OF THE PROPERTY DESCRIBED HEREON, AND RECENTLY WITH SURVEYED WAS MADE UNDER MY RESPONSIBLE, AND CONFORMS WITH THE "STANDARDS OF PRACTICE" AS SET FORTH BY THE FLORIDA BOARD OF PROFESSIONAL LAND SURVEYORS IN CHAPTER 5J-17, FLORIDA ADMINISTRATIVE CODE PURSUANT TO 472.027, FLORIDA STATUTES.

BY: _____ 08/28/2018
JOHN IBARRA (DATE OF FIELD WORK)
PROFESSIONAL LAND SURVEYOR NO. 5204 STATE OF FLORIDA
NOT VALID WITHOUT THE SIGNATURE AND THE ORIGINAL RAISED SEAL OF A FLORIDA LICENSED SURVEYOR AND MAPPER.

REVISED ON: _____
REVISED ON: _____

LEGEND
= OVERHEAD UTILITY LINES
= CONCRETE BLOCK WALL
= CHAIN LINK FENCE
= IRON FENCE
= WOOD FENCE
= BUILDING SETBACK LINE
= UTILITY EASEMENT
= LIMITED ACCESS R/W

DRAWN BY:	KEVIN
FIELD DATE:	08/28/2018
SURVEY NO:	14-000523-10

JOHN IBARRA
PROFESSIONAL LAND SURVEYOR
NO. 5204
STATE OF FLORIDA

JOHN IBARRA & ASSOCIATES, INC.
Professional Land Surveyors & Mappers
WWW.IBARRALANDSURVEYORS.COM
777 N.W. 72nd AVENUE, SUITE 3025, MIAMI, FLORIDA 33126, PH: (305) 262-0400, FAX: (305) 262-0401
4851 TAMIAMI TRAIL NORTH, SUITE # 200, NAPLES, FL 34103, PH: (239) 640-2660, FAX: (239) 640-2664

SITE PLAN

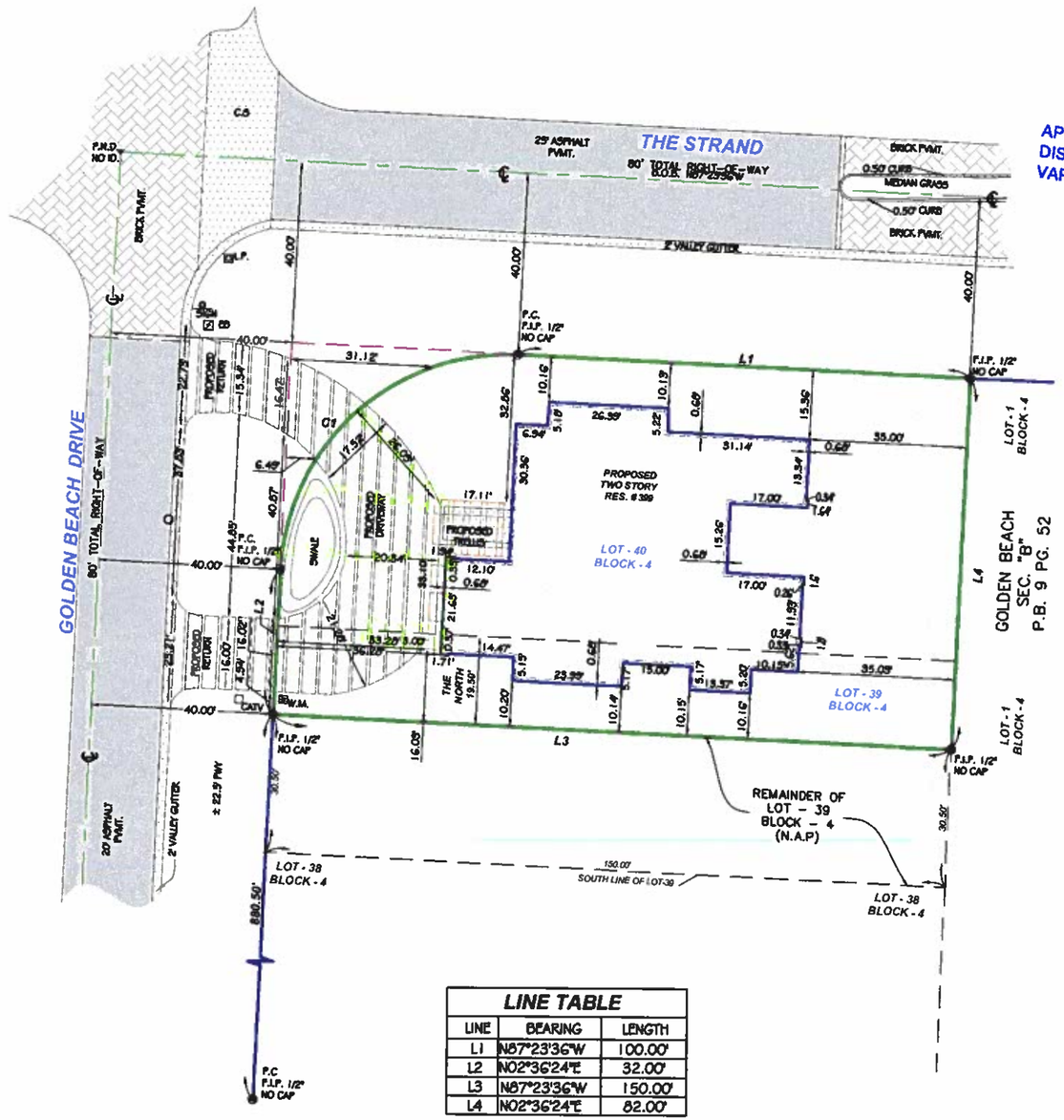
FOR PROPOSED DRIVEWAY AND TRELLIS

GRAPHIC SCALE

-20 0 10 20 40 80

(IN FEET)
1 INCH = 20 FEET

Town of Golden Beach
Building Regulatory Advisory Board
Hearing Date
AUG . 3 2019
APPROVED
DISAPPROVED
VARIANCE REQ:



LINE TABLE

LINE	BEARING	LENGTH
L1	N87°23'36"W	100.00'
L2	N02°36'24"E	32.00'
L3	N87°23'36"W	150.00'
L4	N02°36'24"E	82.00'

CURVE TABLE

CURVE	LENGTH	RADIUS	DELTA	CH. B.	CH. L
C1	78.54'	50.00'	90°00'00"	S47°36'24"W	70.71'

LOCATION SKETCH
SCALE = N.T.S.

LEGAL DESCRIPTION:
THE NORTH 19.50' OF LOT 39 AND ALL OF LOT 40, BLOCK 4, SECTION E OF GOLDEN BEACH, ACCORDING TO THE PLAT THEREOF AS RECORDED IN PLAT BOOK 9, AT PAGE 122, OF THE PUBLIC RECORDS OF MIAMI-DADE COUNTY, FLORIDA.

PROPERTY ADDRESS:
399 GOLDEN BEACH DRIVE,
GOLDEN BEACH, FL 33160

ABBREVIATIONS

A	= ARC
AC	= AIR CONDITIONER PAD
AE	= ARCHER EASEMENT
AL	= ALUMINUM ROOF
ALB	= ALUMINUM SHED
ASPH	= ASPHALT
BC	= BLOCK CORNER
BLDG	= BUILDING
BM	= BENCH MARK
BCR	= RECORDED COUNTY RECORDS
BOB	= BASIS OF BEARING
B.S.L.	= BUILDING SETBACK LINE
C	= CALCULATED
Ch	= CATCH BASIN
C.B.S.	= CONCRETE BLOCK STRUCTURE
C.B.W.	= CONCRETE BLOCK WALL
CB	= CURVE
CPG	= CURVE BEARING
CPL	= CURVE LENGTH
CL	= CLEAR
C.O.	= CLEAN OUT
C.L.F.	= CHAIN LINK FENCE
C.M.E.	= CANAL MAINTENANCE EASEMENT
CONC.	= CONCRETE
C.U.P.	= CONCRETE UTILITY POLE
C.P.	= CONCRETE PORCH
C.S.	= CONCRETE SLAB
C.W.	= CONCRETE WALK
D.E.	= DRAINAGE EASEMENT
D.M.E.	= DRAINAGE MAINTENANCE EASEMENT
DW'WAY	= DRIVEWAY
E	= EAST
EB	= ELECTRIC BOX
E.T.P.	= ELECTRIC TRANSFORMER PAD
ELEV.	= ELEVATION
F.F.	= FINISH FLOOR
F.F.R.	= FOUND IRON PIPE
F.I.R.	= FOUND IRON ROD
F.N.D.	= FOUND NAIL & DISK
F.N.R.	= FOUND NAIL & ROD
FOP	= FOUND OPEN PIPE
F.P.	= FOUND PIPE
F.P.C.	= FOUND PK & DISK
FPK	= FOUND PARKER KALON
G	= GAS
G.M.E.	= GRADING AND DRAINAGE EASEMENT
G.V.	= GATE VALVE
I.F.	= IRON FENCE
L.B.	= LICENSED BUSINESS
L.P.	= LIGHT POLE
L.F.E.	= LOWEST FLOOR ELEVATION
L.M.E.	= LAKE MAINTENANCE EASEMENT
M	= MEASURED
M/B	= MAILBOX
M.E.	= MAINT. EASE.
M.D.C.R.	= MIAMI DADE COUNTY RECORDS
M.E.	= MAINTENANCE EASEMENT
MH.	= MANHOLE
N.A.P.	= NOT A PART OF
NGVD	= NATIONAL GEODETIC VERTICAL DATUM
N.T.S.	= NOT TO SCALE
N	= NORTH
OR	= OFFSET
O.H.	= OVERHEAD
O.H.L.	= OVERHEAD UTILITY LINES
O.R.B.	= OFFICIAL RECORDS BOOK
O.W.M.	= OVERFLOW
PAVE.	= PAVEMENT
PL	= PLANTER
P.L.	= PROPERTY LINE
P.O.C.	= POINT OF COMMENCING SURVEYOR
P.O.C.	= POINT OF CURVATURE
P.O.T.	= POINT OF TANGENCY
P.O.B.	= POINT OF BEGINNING
P.O.B.	= POINT OF BEGINNING
P.R.M.	= POINT OF PERMITTEE COMMENCING
PWY	= PARKWAY
P.R.M.	= PERMANENT REFERENCE MONUMENT
P.S.M.	= PROFESSIONAL LAND SURVEYOR
PP.	= POWER POLE
P.P.E.	= POOL PUMP SLAB
P.U.E.	= PUBLIC UTILITY EASEMENT
RB	= RECORD BEARINGS
R/L	= RAIL ROAD
RES.	= RESIDENCE
R/W	= RIGHT-OF-WAY
R&S	= RADIUS OR RADIAL
RTE.	= ROUTE
R.U.E.	= ROOF OVERHANG EASEMENT
SEC.	= SECTION
STY.	= STORY
SWK.	= SIDEWALK
S.I.P.	= SET IRON PIPE
S	= SOUTH
S.F.	= SQUARE FEET
S.M.E.	= SEWER MAIN EASEMENT
SQ.	= SQUARE
T	= TANGENT
T	= TELEPHONE BOOTH
T.P.E.	= TEMPORARY EASEMENT
T.U.E.	= TELEVISION UTILITY EASEMENT
TEB	= TELEPHONE BOOTH
T.S.B.	= TRAFFIC SIGNAL BOX
T.S.P.	= TRAFFIC SIGNAL POLE
TSP	= TELEPHONE
UTIL.	= UTILITY
U.E.	= UTILITY EASEMENT
U.P.	= UTILITY POLE
W.M.	= WATER METER
W.F.	= WOOD FENCE
W.P.	= WOOD POWER
W.R.	= WOOD ROOF
W.V.	= WATER VALVE
W	= WEST
℄	= CENTER LINE
Δ	= DELTA

LEGAL NOTES TO ACCOMPANY SKETCH OF SURVEY :
- THERE MAY BE EASEMENTS RECORDED IN THE PUBLIC RECORDS NOT SHOWN ON THIS SURVEY.
- EXAMINATIONS OF THE ABSTRACT OF TITLE WILL HAVE TO BE MADE TO DETERMINE RECORDED INSTRUMENTS, IF ANY, AFFECTING THE PROPERTY.
- THIS SURVEY IS SUBJECT TO DEDICATIONS, LIMITATIONS, RESTRICTIONS, RESERVATIONS OR EASEMENTS OF RECORD.
- LEGAL DESCRIPTIONS PROVIDED BY CLIENT OR ATTESTING TITLE COMPANY.
- BOUNDARY SURVEY MEANS A DRAWING AND/OR A GRAPHIC REPRESENTATION OF THE SURVEY WORK PERFORMED IN THE FIELD, COULD BE DRAWN AT A SHOWN SCALE AND/OR NOT TO SCALE; THE WALLS OR FENCES MAY BE EXAGGERATED FOR CLARITY PURPOSES.
- EASEMENTS AS SHOWN ARE PER PLAT BOOK, UNLESS DEPICTED OTHERWISE.
- THE TERM "ENCROACHMENT" MEANS VISIBLE AND ABOVE GROUND ENCROACHMENTS.
- ARCHITECTS SHALL VERIFY ZONING REGULATIONS, RESTRICTIONS, SETBACKS AND WILL BE RESPONSIBLE FOR SUBMITTING PLOT PLANS WITH CORRECT INFORMATION FOR "APPROVAL FOR AUTHORIZATION" TO THE PROPER AUTHORITIES IN NEW CONSTRUCTION.
- UNLESS OTHERWISE NOTED, THIS FIRM HAS NOT ATTEMPTED TO LOCATE FOOTING AND/OR FOUNDATIONS.
- FENCE OWNERSHIP NOT DETERMINED.
- THIS PLAN OF SURVEY, HAS BEEN PREPARED FOR THE EXCLUSIVE USE OF THE ENTITIES NAMED HEREON, THE CERTIFICATE DOES NOT EXTEND TO ANY UNNAMED PARTY.

FLOOD ZONE INFORMATION:
THE NFIP FLOOD MAPS HAVE DESIGNATED THE HEREIN DESCRIBED LAND TO BE SITUATED IN:

FLOOD ZONE:	"AE"
BASE FLOOD ELEVATION:	6 FT.
COMMUNITY:	120079
PANEL:	0163
SUFFIX:	L
DATE OF FIRM:	09/11/2009

THE SUBJECT PROPERTY DOES LIE IN A SPECIAL FLOOD HAZARD AREA.

SURVEYOR'S NOTES:
1. IF SHOWN, BEARINGS ARE REFERRED TO AN ASSUMED MERIDIAN, BY SAID PLAT IN THE DESCRIPTION OF THE PROPERTY, IF NOT, BEARINGS ARE THEN REFERRED TO COUNTY, TOWNSHIP MAPS.
2. THE CLOSURE IN THE BOUNDARY SURVEY IS ABOVE 1:7500 FT.
3. CERTIFICATE OF AUTHORIZATION LB # 7806.

SURVEYOR'S CERTIFICATION:
I HEREBY CERTIFY THAT THE HEREON SURVEY OF THE PROPERTY DESCRIBED HEREON, HAS RECENTLY BEEN SURVEYED AND DRAWN UNDER MY SUPERVISION, AND COMPLIES WITH THE STANDARDS OF PRACTICE AS SET FORTH BY THE FLORIDA BOARD OF PROFESSIONAL LAND SURVEYORS IN CHAPTER 61G17, FLORIDA ADMINISTRATIVE CODE PURSUANT TO 472.027, FLORIDA STATUTES.

07/17/2019
BY: JOHN IBARRA — DATE OF FIELD WORK

PROFESSIONAL LAND SURVEYOR NO.: 5204 — STATE OF FLORIDA
NOT VALID WITHOUT THE SIGNATURE AND THE ORIGINAL RAISED SEAL OF A FLORIDA LICENSED SURVEYOR AND MAPPER.

REVISED ON: 07/17/2019 SITE PLAN FOR PROPOSED TRELLIS
REVISED ON: 06/28/2019 SITE PLAN FOR PROPOSED DRIVEWAY
REVISED ON: 04/29/2019 FORM BOARD SURVEY
REVISED ON: 03/01/2017 UPDATED WITH CONSTRUCTION ELEVATION
REVISED ON: 04/13/2016 ADDED TREES
REVISED ON: 01/15/2016 UPDATED WITH CONSTRUCTION ELEVATION
REVISED ON: 03/13/2014 ORIGINAL SURVEY

LEGEND

- O-H = OVERHEAD UTILITY LINES
- = CONCRETE BLOCK WALL
- = CHAIN LINK FENCE
- = IRON FENCE
- = WOOD FENCE
- = BUILDING SETBACK LINE
- = UTILITY EASEMENT
- = LIMITED ACCESS R/W
- = NON-VEHICULAR ACCESS R/W
- x 0.00 = EXISTING ELEVATIONS

DRAWN BY:	LK
FIELD DATE:	07/17/2019
SURVEY NO:	14-000523-16
SHEET:	1 OF 1

JOHN IBARRA R.P.S.M. — CERTIFICATE — PROFESSIONAL LAND SURVEYOR — NO. 5204 STATE OF FLORIDA
L.B.# 7806 SEAL

JOHN IBARRA & ASSOCIATES, INC.
Professional Land Surveyors & Mappers
WWW.IBARRALANDSURVEYORS.COM
777 N.W. 72nd AVENUE
SUITE 3025
MIAMI, FLORIDA 33126
PH: (305) 262-0400
FAX: (305) 262-0401

2004 CIEL PRADO BLVD SOUTH
SUITE NO. 202 UNIT 1
CAPE CORAL, FL 33904
PH: (239) 540-2660
FAX: (239) 540-2664

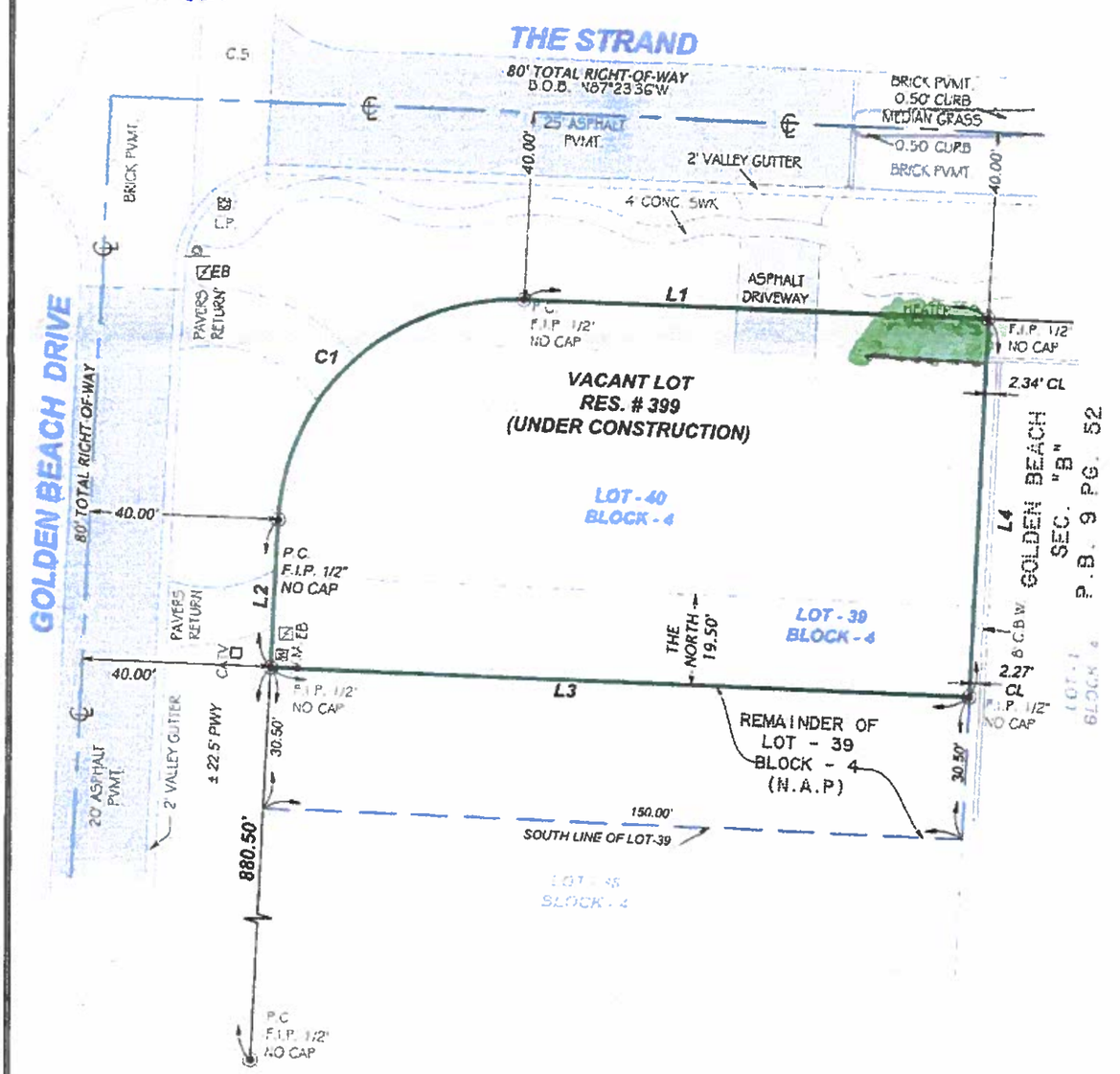
MAP OF BOUNDARY SURVEY

399 GOLDEN BEACH DRIVE, GOLDEN BEACH, FL 33160

Town of Golden Beach
Building Regulatory Advisory Board
Hearing Date AUG 13 2018
APPROVED
DISAPPROVED
VARIANCE REQ:

LINE TABLE		
LINE	BEARING	LENGTH
L1	N87°23'36"W	100.00'
L2	N02°36'24"E	32.00'
L3	N87°23'36"W	150.00'
L4	N02°36'24"E	82.00'

CURVE TABLE					
CURVE	LENGTH	RADIUS	DELTA	CH. B.	CH. L
C1	78.54'	50.00'	90°00'00"	S47°36'24"W	70.71'



CARLOS IBARRA CERTIFICATE NO. 6770 STATE OF FLORIDA PROFESSIONAL LAND SURVEYOR
L.B.# 7806 SEAL

LEGAL DESCRIPTION:
THE NORTH 19.50' OF LOT 39 AND ALL OF LOT 40, BLOCK 4, SECTION E OF GOLDEN BEACH, ACCORDING TO THE PLAT THEREOF AS RECORDED IN PLAT BOOK 8, AT PAGE 122, OF THE PUBLIC RECORDS OF MIAMI-DADE COUNTY, FLORIDA.

CERTIFICATION:
GALSKY CONSTRUCTION ENTERPRISES 2, LLC, A FLORIDA LIMITED LIABILITY COMPANY
HOWARD B. NADEL, P.A.
OLD REPUBLIC NATIONAL TITLE INSURANCE COMPANY
INTERAUDI BANK, ISAOA, ATIMA

DRAWN BY:	EA
SCALE:	1"=30'
FIELD DATE:	01/04/2018
JOB NO:	14-000523-5
SHEET:	2 OF 2

Linda Epperson

From: Melinda Almonte <scarveyalmonte@bellsouth.net>
Sent: Wednesday, July 31, 2019 3:46 PM
To: Linda Epperson
Cc: Alexander Diaz
Subject: 399 Golden Beach Drive Variance

Dear Ms. Epperson and Mr. Diaz, This is Melinda Almonte at 395 Golden Beach Drive, the south neighbor of 399 Golden Beach Drive. I am absolutely opposed to the variance requested by the owner of the property. It will be a large eye sore at the entrance of our community infringing on 9 feet of land that should have a setback on the north curve and almost 2 feet on the south front side. I don't accept that infringement. The house structure itself is already at the maximum building capacity for setback requirements. It is not a good look for the town to then violate the setback rules. We start to look like a community of large town homes. I object on my own behalf and for other residents who follow the building codes. Sincerely, Melinda Almonte

Linda Epperson

From: Melinda Almonte <scarveyalmonte@bellsouth.net>
Sent: Friday, August 02, 2019 3:35 PM
To: Linda Epperson
Cc: Alexander Diaz
Subject: 399 Golden Beach Drive Variance Request

As an addendum to my prior email opposing the owner's request for a variance for the above-referenced property, I would like to emphasis that there is no showing of good and sufficient cause nor an exceptional hardship for the owner that would result if the variance is not granted. These conditions must be met under Golden Beach Code of Ordinances Article II, Division 4, Section 62-51 to qualify for a variance. The purpose of a variance is to grant equitable relief to a property owner where the physical characteristics of the property, i.e. shape of the lot or other natural characteristics of the property, severely affect the owner's ability to build on the property without a variance. That is clearly not the case here. I propose the query as to what the neighborhood would look like if everyone wanted to build structures in their front yard that violated the setback requirements?
Thank-you, Melinda Almonte, Esq.

Linda Epperson

From: Melinda Almonte <scarveyalmonte@bellsouth.net>
Sent: Wednesday, July 31, 2019 3:46 PM
To: Linda Epperson
Cc: Alexander Diaz
Subject: 399 Golden Beach Drive Variance

Dear Ms. Epperson and Mr. Diaz, This is Melinda Almonte at 395 Golden Beach Drive, the south neighbor of 399 Golden Beach Drive. I am absolutely opposed to the variance requested by the owner of the property. It will be a large eye sore at the entrance of our community infringing on 9 feet of land that should have a setback on the north curve and almost 2 feet on the south front side. I don't accept that infringement. The house structure itself is already at the maximum building capacity for setback requirements. It is not a good look for the town to then violate the setback rules. We start to look like a community of large town homes. I object on my own behalf and for other residents who follow the building codes. Sincerely, Melinda Almonte